WATER POLLUTION CONTROL AUTHORITY

Town of Trumbull

CONNECTICUT

TOWN HALL
(203) 452-5048

5866 MAIN STREET
TRUMBULL, CT 06611

FEBRUARY 26, 2026
MINUTES

CALL TO ORDER: The Chair called the meeting to order at 6:01 p.m.

ROLL CALL: The clerk called the roll and recorded it as follows:

PRESENT: Chairman Frank Regnery, Vice-Chairman John Bryk, Secretary Jon Greene,

ABSENT: Bruce Elstein

Also Present: WPCA Administrator/Town Engineer William Maurer, Town Attorney James Nugent and Christine Kurtz of Wright Pierce

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DISCUSSION ITEMS:

- February 21, 2026, Pump Stations Tour – Bill Maurer reported the pump station tour was conducted on Saturday, February 26, 2026, with newer members of the Board of Finance and Town Council. The tour included visits to Beardsley, 135 Merritt Boulevard, Reservoir, and Wildwood pump stations. The concrete issues at the Beardsley pump station were pointed out during the tour.

- Bridgeport WPCA & Town of Trumbull Agreement – Bill Maurer informs the commission that the contract with Bridgeport runs out on June 30, 2026, and there are three more years to extend it. Attorney Nugent suggested putting Bridgeport on immediate notice of the extension and that a new agreement will likely need to be negotiated.

- Earmark Grant $1.1 Million Award from the Office of Jim Himes – Bill Maurer explained the WPCA has been awarded a grant of $1.1 million for the next section of Beardsley force main replacement under the Pequannock River and Route 25. Once the funding is made available, they will move forward with the design.

NEW BUSINESS:

- Wildwood Pump Station Emergency Repair Approval – Bill Maurer explained that the pump station services a large area, including a middle school, and that they lost one of the two pumps and there are no back-up pumps. Bill Maurer shared his screen showing

pictures of what needed to be repaired. This is one of three pump stations being currently redesigned. Arcadis is doing the Q&A for the bid documents now and we should have final bid documents by the end of March. Bill Maurer shared the Kovacs invoice for the new base and rewiring of the old pump totaling $14,249.62with the commission. After further discussion the Chair called for a motion and a second to approve.

Moved by Bryk, seconded by Greene and a second to approve the Kovacs Construction Invoice #F-3035, dated 2-20-2026 in the amount of $14,249.62. (Invoice attached). VOTE: Motion CARRIED by unanimous consent.

- JANUARY 14, 2026, MEETING MINUTES APPROVAL: https://www.trumbull-ct.gov/AgendaCenter/ViewFile/Minutes/_01142026-5961 Moved by Greene, seconded by Bryk to approve the January 14, 2026, minutes as submitted. VOTE: Motion CARRIED by unanimous consent

- FY2026-2027 Operating Budget Update Approval – Bill Maurer reviewed the FY 2026-2027 operating budget in detail with the commission. He highlighted salaries, professional fees, and the rate with Bridgeport. The budget includes a 3% rate increase from Bridgeport and a potential payment for excess flow. The commission noted $50,000 is too low for unexpected repairs.

  Moved by Greene, seconded by Bryk to approve the WPCA Fiscal Year 2026-2027 Operating Budget as submitted. VOTE: Motion CARRIED by unanimous consent.

OLD BUSINESS:

- Update - Trumbull WPCA 2026 Capital Improvement Plan (CIP)-$2,700,000 Bond Issuance – Bill Maurer explained the bond issuance was approved at the council level and noted the capital plan includes funds for force main design and gravity line repairs and was approved as submitted. This is for calendar year 2026 and the funds are available now.

- 2135-2145 & 2155 Reservoir Avenue Sanitary Sewer Connection Request – Attorney Jason Klein representing Vessel Technologies and Kevin Solli of Solli Engineering were present, as wells Josh Levy Vice President of Vessel Technologies.

  Kevin Solli shared his screen. The following was reviewed, (slide presentation & additional supplemental material submitted prior to this meeting by Vessel Technologies attached), and points of discussion are as follows:
  - High level calculations based on empirical data from a similar project in New London, showing a reasonable flow estimate, see attached).
  - The proposed project would add 14,560 gallons per day to the existing flow, bringing the total to 133,000 gallons per day.
  - Solli Engineering suggested installing a holding tank to manage peak flows, estimating the cost at $200,000. The tank would hold 2-3 days of flow.
  - It was confirmed there are 1 and 2-bedroom apartments

- The Chair expressed concerns about the capacity of the existing eight-inch pipe and suggested replacing it with a ten-inch pipe.
- Christine Kurtz noted a 10-inch pipe would significantly improve capacity and Bill Maurer spoke in favor of replacing the pipe rather than using a holding tank.
- Bill Maurer estimated the cost of replacing the eight-inch pipe to a ten-inch pipe at $720,000. Bill Maurer confirmed this was a good estimate.
- The Chair proposed that Vessel Technology contribute $400,000 towards the pipe replacement.
- Vessel Technologies agrees to the contribution and suggests reconvening to finalize the details if the town can confirm the funds available.
- Attorney Klein suggested confirming there is money in the budget for the WPCA's portion of the cost of the pipe replacement before taking formal action.
- Bill Maurer stated he was confident WPCA would have funding for the remaining cost of the pipe replacement.

Moved by Greene, seconded by Bryk to the sanitary sewer connection as submitted with the following conditions:

- The developer will contribute $400,000 to increase the sanitary sewer pipe size from 8" to 10".
- The Town's portion of funding to increase the size of the sanitary sewer pipe be found in the operating budget or the bonding be approved by the Board of Finance and Town Council.
- The developer will pay the developer installed unit fee, (connection fee) of $1,050,000 (150 apartments x $7,000= $1,050,000).
- The approval is in effect for 24 months, provided that within that time, a building permit is obtained.

VOTE: Motion CARRIED by unanimous consent.

Josh Levy of Vessel Technologies extended his gratitude for how collaborative the commission has been, and Attorney Jay Klein noted how accessible staff and the town's attorney has been throughout this process.

The Chair thanked the commission for their hard work.

ADJOURNMENT: There being no further business to discuss, and upon motion made by Bryk and seconded by Greene the Trumbull WPCA adjourned at 7:38 by unanimous consent.

Respectfully submitted,

Margaret D. Mastroni

Margaret D. Mastroni, clerk

KOVACS CONSTRUCTION CORP.

321 RIGGS STREET OXFORD, CT 06478-1129
TELEPHONE (203) - 743 - 4022 FAX (203) - 790 - 1326

INVOICE

INVOICE DATE	CUSTOMER ID	INVOICE #	P.O. #	TERMS
02-20-2026	TRUMBULLTO	F-3035		Net 30

TO:
Town of Trumbull
5866 Main St.
Trumbull, CT 06611

JOB LOCATION:
TRUMBULL-WILDWOOD PS

A FINANCIAL CHARGE OF 1½% (ANNUAL RATE 18%) will be charged on unpaid balance past due after 30 days. If collection action and used to enforce any terms, attorney's fees will be added to the account.

Description	Quanity	Measure	Unit Price	Total
RE: Wildwood Pump Station- Install new pump in Can station				
2/4/2026				
Went to site to make a list of materials needed to install new pump.	4.00	hr	140.00	560.00
2/5/2026				
Load Materials, tools, etc. Went to site. Remove old base and 90 suction, removed 8X10 90 and rotate 45 degrees and rebolt. Rig and set pump base and lower in to pump staton. Rig and lower 6" composite hose into pump station and bolt up. Block up 6" suction 90 with lumber and take measurements for welding legs on pump base and attached to steel floor.	29.00	hr	140.00	4,060.00
2/6/2026				
Went to josite. Weld pump support to floor and weld hose support to floor. Install motor.	24.00	hr	140.00	3,360.00
2/9/2026				
Went to jobsite. Check pump operation and cleaned up.	6.00	hr	140.00	840.00
Materials				
Two (2) 6" FF gaskets	2.00	ea,	25.00	50.00
(24) 3/4" X 3-1/2" black bolts and nuts	24.00	ea.	5.36	128.64
One (1) 6X4" DI flg 90 elbow	1.00	ea	513.80	513.80
One (1) 6" X 10' composite hose float X fixed flange	1.00	ea.	1,785.00	1,785.00

TOTAL DUE THIS INVOICE

KOVACS CONSTRUCTION CORP.

321 RIGGS STREET OXFORD, CT 06478-1129
TELEPHONE (203) - 743 - 4022 FAX (203) - 790 - 1326

INVOICE

INVOICE DATE	CUSTOMER ID	INVOICE #	P.O. #	TERMS
02-20-2026	TRUMBULLTO	F-3035		Net 30

TO:
Town of Trumbull
5866 Main St.
Trumbull, CT 06611

JOB LOCATION:
TRUMBULL-WILDWOOD PS

Description	Quanity	Measure	Unit Price	Total
8' C-channel 6X2X 5/16"	1.00	ea.	183.33	183.33
2' 3X3 x3/8" angle iron	2.00	ft	14.98	29.96
Welder, rod, gas etc.	1.00	lot	372.00	372.00
Transportation				
Three (3) Round trips	3.00	ea.	120.00	360.00
Subcontractor				
Electrical Wiring of new Pump	1.00	ea.	2,006.89	2,006.89

A FINANCIAL CHARGE OF 1½% (ANNUAL RATE 18%) will be charged on unpaid balance past due after 30 days. If account is placed with attorneys for collection, attorneys fees will be added to the amount.

AMOUNT BILLED	$14,249.62
TOTAL TAX	
RETAINAGE HELD	
TOTAL DUE THIS INVOICE	$14,249.62

February 25, 2026

Mr. William Maurer
Trumbull Town Engineer
5866 Main Street
Trumbull, CT 06611

RECEIVED
FEB 25 2026
WATER POLLUTION CONTROL AUTHORITY
BY

Re: Supplemental WPCA Materials
Trumbull Vessel
2135-2155 Reservoir Avenue
Trumbull, CT 06611

Dear Mr. Maurer,

On behalf of Vessel Technologies, please find enclosed a hard copy of supplemental WPCA submission materials for the proposed Vessel multifamily project at 2135-2155 Reservoir Avenue. We look forward to the continued discussion with the WPCA. If you have any comments or questions, please fee to reach out to me at luke@sollillc.com or at 203-880-5455.

Respectfully,

Solli Engineering, LLC

Luke Mauro, P.E., PTOE
Senior Project Manager

Enclosures:
- Copy of Trumbull Code of Ordinances Section 13 – Water Pollution Control Authority
- Response to WPCA Engineering Comments, prepared by Solli, dated January 20, 2026
- Letter from Carmody to Trumbull WPCA, regarding Sanitary Sewer Connection Request, dated February 10, 2026
- Professional Resume for Luke Mauro
- WPCA Meeting Presentation Slide Deck for February 25, 2026 meeting, prepared by Solli
- Alternate Utility Plan (Sheet 2.51), prepared by Solli, dated February 11, 2026

X:\SE Files\Project Data\2024\24116901 - 2135, 2145, 2155 Reservoir Ave - Trumbull, CT\Archive\Outbound Archive\Submissions\Applications\2026-02-25 WPCA Submission\Trumbull Vessel WPCA Cover Letter 2026-02-25.docx

Section 13. - Water Pollution Control Authority.

A. *Composition and appointment.* The Town shall have a Water Pollution Control Authority which shall consist of five (5) members and two (2) alternate members who shall be appointed by the First Selectman. Each member shall serve for a term of five (5) years. Each alternate member shall serve for a term of two (2) years. The term of one (1) member shall expire each year. The two alternate members shall not both be members of the same political party.

B. *Powers and duties.* The Water Pollution Control Authority shall have all the powers and duties conferred or imposed by law on Water Pollution Control Authorities.

RECEIVED

FEB 25 2026

WATER POLLUTION
CONTROL AUTHORITY

BY _____

Response to WPCA Engineering Comments

To: William Maurer, P.E., L.S. / Trumbull Town Engineer
Christine Kurtz, P.E. / Wright-Pierce
Joe Hausmann / Wright-Pierce

From: Luke Mauro, P.E., PTOE
Kevin Solli, P.E., PTOE

Subject: Proposed Vessel Multifamily Development
2135, 2145 & 2155 Reservoir Ave, Trumbull, CT
Project #: 24116901

Date: January 20, 2026

RECEIVED
FEB 25 2026
WATER POLLUTION CONTROL AUTHORITY
BY _____

The following is in response to sewer review comments for the above referenced project, from Wright-Pierce dated December 18, 2025. We would note that while the comments are dated 12/18/25, we were not in receipt of the comment memo until 1/7/26. A response to each comment is provided in bold when applicable.

We provide the following comments on the sewer flow estimate provided by the applicant:

1) The reviewed plans show the proposed installation of approximately 160 LF of 8" sewer from the road to an first manhole on site, and an additional 525 LF of 6" sewers from the first manhole (labeled as MH-6) on the site to the furthest manhole on the site (labeled as MH-1).

   Noted. No response required.

2) An email from the Engineer provided to us by the WPCA presented an average daily flow (based on estimated number of residents) as well as a peak hourly flow based on the average daily flow and a peaking factor of 1.5.

   a. They requested an average daily flow of 26,700 gpd, and a peak flow of 40,050 gpd.

      Noted. No response needed.

   b. Other than the breakup of units by types (1 bedroom, 2 bedroom, and 2 bedroom with additional den) there was no backup provided on how the flows were estimated.

      i. It appears they utilized 150 gpd/bedroom in accordance with the CT public health design codes; this is acceptable for wastewater flows. However, there was no I/I allowance assumed for this proposed pipe within the estimated flow calculations.

      A design flow of 150 gpd/bedroom was utilized for the initial estimates for the average daily sewer flow based on guidance provided by engineering department. 150 gpd/bedroom x 178 bedrooms = 26,700 gpd for the entire development.

      When available, we typically use empirical data for calculating design flows and in the absence of empirical data we either rely on guidance from the town engineer or adhere to the design guidance in the 2024 CT DPH "On-site Sewage Disposal Regulations and Technical Standards for Subsurface Sewage Disposal Systems" (Technical Standards), which is accepted as the standard for the design of septic systems in the state of Connecticut. Per Section IV – Design Flows in the Technical Standards, recommended design flows are 150 GPD per bedroom for the first three bedrooms and then 125 GPD per bedroom for additional bedrooms beyond the first three for multifamily buildings. This would equate to 10,575 gpd for the eastern 84-bedroom building (150 x 3 + 125 x 81 = 10,575) and 11,825 gpd for the western 94-bedroom building (150 x 3 + 125 x 91 = 11,825). Based on the latest Technical Standards the total average daily sewer flow would be estimated at 22,400 gallons per day, which is *4,300 gpd less*, or *16% lower* than the 26,700 gpd utilized based on guidance given by the engineering department.

      For reference, the guidance in the town of Bethel is to utilize average daily flows of 100 gpd for multifamily developments. For multifamily applications in the town of Milford, there is a set design criteria indicating efficiency (studio) units have a design flow of 60 gpd, single bedroom apartments have a design flow of 75 gpd, and each additional bedroom is assigned a design flow of 40 gallons per day, again with no peaking factor assigned to the average daily flow.

Vessel has provided empirical usage data for their multifamily development consisting of 30 single bedroom units in New London, CT. Water Bills from the New London Department of Public Utilities are attached depicting average daily usage from 2/27/2025 to 11/17/2025, when the building was fully leased. A total of 34 individuals live in the building, equating to an occupancy of 1.13 people per bedroom or per unit in this case, since all units are single bedrooms. Over this period the average daily usage for the building was 986 gallons-per-day (gpd), which equates to 32.86 gpd per bedroom or 29 gpd / occupant. The period of highest usage was the billing cycle between 2/27/25 and 6/2/25, where the average daily usage for the building was 1160 gpd, or 38.66 gpd per bedroom and 34.12 gpd per occupant.

Based on data referenced later in this review memo, Wright-Pierce indicates that for the 20 lots being connected to the public sewer system along Eddie Road, Rocky Hill Terrace and Garland Circle, an average daily flow rate of 70 gpd per person was applied, which is significantly higher than the empirical data provided by Vessel for the New London building. An assumption was made that each of the lots had 2.8 residents. While the exact house that were to be connected to public sewer is unknown, a review of the tax cards indicates the following:

- **There are 5 houses on Eddie Road each containing 3 bedrooms**
- **There are 11 houses on Garland Road, one 2-bedroom, eight 3-bedroom and two 4-bedroom**
- **There are 14 houses on Rocky Hill Terrace, nine 3-bedroom and five 4-bedroom**
- **Total 30 houses and 96 bedrooms, or an average of 3.2 bedrooms per house or lot**

Applying the 2.8 residents per lot to the average of 3.2 bedrooms per lot, equates to an occupancy of *0.875 residents per bedroom* (2.8/3.2 = 0.875) for the houses in the adjacent neighborhood. This is *23% lower* than Vessel's empirical data of 1.13 occupants per bedroom from New London.

The flow rate applied by Wright-Pierce for this neighborhood was 196 gpd per lot (2.8 occupants x 70 gpd per occupant = 196 gpd). At an average of 3.2 bedrooms per lot, that equates to only *61.25 gpd per bedroom*. This rate is *88.75 gpd lower or 59% lower* than the 150 gallons per bedroom guidance given by the engineering department. There is an inconsistency between how the flow rate is applied for the Rocky Hill Terrace neighborhood and the Vessel development, and the Rocky Hill Terrace average flow rates are *significantly lower* than the standards outlined in the CT Public Health Code. Had this data been known before our initial submission, we would have recommended a lower, more appropriate average daily flow per bedroom as part of our original calculations.

Utilizing the same average occupancy of 1.13 occupants per bedroom in New London, this would equate to a total occupancy of 202 people for the proposed 178 bedrooms for the Trumbull Vessel project. At 70 gpd per person, the total estimated average daily flow for Trumbull Vessel would be 14,140 gpd, or 94.27 gpd per bedroom. This is over *50% higher* than the 61.25 gpd per bedroom applied for the Rocky Hill Terrace project. While conservative and significantly higher than Vessel's empirical data and the design rates used for Rocky Hill Terrace, the 14,140 gpd is a more realistic estimate of the average daily flow for this project. *With a conservative infiltration rate of 420 gpd included, that totals 14,560 gpd.*

ii. For the purposes of our review, we have included an infiltration allowance of 500 gpd/in-mile for the assumed 6" and 8" pipe between the proposed saddle connection in the road and proposed MH-1 on the site. This adds an additional 420 gpd to the average daily flow. But this flow is not peaked for the purposes of peak flow analysis.

Noted; however, based on a review of the 2011 Edition of TR-16 "Guides for the Design of Wastewater Treatment Works" by NEIWPCC, section 2.2.3.3 - Infiltration states: "Add an appropriate allowance to the capacity of sanitary sewers for infiltration due to normal aging of piping systems. An allowance of 250–500 gpd/in. diameter/mile of sewer (0.24–0.48 m3/cm of pipe diam/km/day) is suggested as a normal range of infiltration." We would note that the most conservative value of 500 gpd/in-mile was selected and applied throughout this memo, which is highly unlikely given that all the developments referenced, including the Vessel development, would be utilizing brand new PVC piping not subject to extensive infiltration. While we agree it is appropriate to assign some infiltration factor for older, deteriorated piping, it is not realistic to assume that the new PVC piping will experience any significant infiltration volume.

While we do not know the actual length of pipe runs within the entire network, based on an assumption of 3 miles of pipe up to the conflict area in Reservoir Avenue and an assumed pipe size of 8" for the entire network, the infiltration flow rate for the entire system would only be 12,000 gpd (500 x 3 x 8 = 12,000) under high precipitation conditions. This potential 0.012 MGD is negligible compared to the entire flow within the system and would account for less than 2% of the pipe capacity of 0.77 MGD.

c. The peaking factor for peak flows does not meet the requirement for how peak flow is calculated in Trumbull, following state standards. The applicant references a calculation / approach taken by a similar proposed development in this same area; note the WPCA rejected that calculation.

In our experience working with various WPCAs and Sewer Commissions, this is atypical. In making numerous WPCA applications, including but not limited to Bridgeport, Milford, Shelton, Orange, Newtown, Bethel, Glastonbury and The Metropolitan District Commission (MDC), we have not been asked to identify a peaking factor to average daily sewer flow estimates and have also not been notified that these agencies have applied a peaking factor to our average daily sewer flow estimates when evaluating capacity for our development flows. We recommended a peaking factor of 1.5, similar to the 2105 Reservoir Road development, as our understanding was that a value for a potential peak flow rate over an entire day was being requested.

Most recently in Trumbull, Solli submitted an application for a medical office building at 6600 Main Street in February 2025. Based on our recollection of the review process for that application and to our knowledge, a peaking factor was not applied or discussed as part of that review.

i. The proposed flows anticipated from this development along with the estimated flows along this section of Reservoir Avenue, per the guidance in TR-16, we recommend using a peaking factor of 5.6 be applied to average day flow to estimate the peak flow during a maximum day. This is the flow used to model peak flow scenarios to ensure no adverse effects.

We note that the TR-16 is a guidance document and not a code requirement and should be applied appropriately with consideration given to empirical data. The introduction to the TR-16 states: "...As a guide, it is not to be construed as superseding the requirements, regulations, policies, and standards of the appropriate state water pollution control agencies. Users of this guide should be aware of all applicable regulations, and local and state regulators should always be contacted before starting facility planning. Users should also be familiar with federal requirements that apply to the development of engineering reports, plans, and specifications."

Section 2.2.4 - Peak Design Flow of TR-16 states: "Where existing data are unavailable, peak domestic design flow may be determined using the ratio of peak to average daily flow as described in Figure 2-1 (from the Water Environment Federation/American Society of Civil Engineers Manual of Practice (WEF/ASCE) publication, MOP-FD-5 Gravity Sanitary Sewer Design and Construction)." Additionally, in Section 1.2.4 – Flow Rates of TR-16 one of the bulleted points of information to include in an engineering report for a wastewater treatment system is "Per capita wastewater average distribution. Calculate these flow rates using existing state data, information from technical journals, and/or actual water use rates." The guidance in the TR-16 is clear that empirical data should be utilized as the best practice to calculate peak flow rates in an existing sewer treatment system.

Based on our understanding, the methodology and calculations used in this memo are not based on actual individual or systemic average daily flows or peak flows within the sewer system in Reservoir Drive. It appears that they are based on a model which typically applies the most conservative 5.6x peaking factor each time a new development is added to the model/calculations. As a result, it appears that this methodology assumes that all new developments are peaking at a rate at 5.6x their average daily flow rate and that they are also peaking at the exact same time. We note that the 5.6x peaking factor is recommended for the *maximum hourly flow on the heaviest day of the year* for the lowest potential average daily flows, accounting for a very large variance in typical flow rates for very small sewer generators. Applying this peaking factor is extremely conservative and has the potential to identify capacity issues in a sewer network that may not actually exist but are theoretical. We note again that TR-16 is a guidance document and not an actual code or ordinance that requires strict enforcement.

Data provided later in this memo states that the capacity model accounts for 0.66 MGD as the peak existing flow within the Reservoir Avenue sewer system. This includes 0.023 MGD for the 20 lots off Rocky Hill Terrace and 0.052 MGD for the approved development at 2105 Reservoir Avenue, with the 5.6x peaking factors applied to each. That said, the base model for Reservoir Avenue excluding these developments would be flowing at a rate of 0.585 gpd (0.66 – 0.023 – 0.052 = 0.585). Assuming the 5.6x peaking factor was applied to arrive at the 0.585 gpd peak flow with Reservoir Road, the average base flow would be 0.105 MGD. Adding in the approved flows of 3,920 gpd for the 20 lots off Rocky Hill Terrace, the 9,490 gpd for 2105 Reservoir Road and the 14,560 gpd flow to be generated by the Vessel development, totals 0.133 gpd for an average daily flow rate within Reservoir Road. Per Figure 2-1 in the TR-16, at this average flow rate of 0.133 gpd, the recommended peaking factor on the maximum day would be 5.2x the average daily flow for this segment of the system within Reservoir Avenue. Again, this factor taken from Figure 2-1 accounts for a potential peak flow rate during the

highest hour during the heaviest day and should not be construed as a typical system operation, which is why empirical data is recommended by TR-16 to be used as the basis for sewer design.

For the numerous reasons outlined above, the application of this 5.6x peaking factor on a project-by-project basis and holistically throughout the sewer system model doesn't seem appropriate. Empirical data specifically for this section of Trumbull is more appropriate to use to determine a more accurate average daily flow and peak flow within the sewer system in Reservoir Avenue.

3) Based on the above recommendations, for the purposes of our capacity analysis, we utilized the following flows:
   a. Average Daily flow of 27,120 gpd. (26,700 gpd as requested plus 420 gpd as an infiltration allowance)
   b. Peak flow of 149,940 gpd as a rate (26,700 avg gpd times 5.6 the peaking factor, plus 420 gpd for Infiltration).

As outlined in our responses above, a more appropriate, yet still conservative, average daily flow of 14,560 gpd should be utilized. This is based on the 70 gpd per person guidance from TR-16 and also as applied by Wright-Pierce for the 20-lot sewer connection project for Rocky Hill Terrace (outlined below). Applying a conservative peaking factor of 5.2 from Figure 2-1 of TR-16, for the maximum hour on the heaviest day of the year, this would result in a maximum peak flow of 73,950 gpd including an additional conservative estimate for pipe infiltration (14,140 x 5.2 + 420 gpd infiltration allowance).

We provide the following comments as a summary of evaluations done for this portion of the sewer system, including our updates to the sewer capacity model downstream of the proposed development:

4) We utilized the sewer capacity analysis from previous developments along Reservoir Avenue as the base information for this analysis.
   a. This included reviews of developments at 2300 Reservoir Avenue and 100 Oakview Drive (off of Lindeman Drive) in previous years, as well as the recent proposal of a development at 2105 Reservoir in October of 2025. These reviews included the existing buildout of the entirety of the connected properties at that time.

Noted. It appears that 2300 Reservoir Avenue and 100 Oakview Drive were constructed and are actively contributing flows to the sewer system. We request clarification as to whether the flows associated with these developments were input into the sewer analysis model using their design flows or actual measured flow data/water usage data provided after opening. The applicant has indicated they are willing to install flow meters within the Reservoir Avenue sewer system to identify typical average day and peak flow rates within the system.

5) Several years ago, the analysis of the sewer capacity associated with the proposed development at 2300 Reservoir Avenue identified a segment of pipe that was undersized.
   a. This pipe was replaced with an increased size as part of the development of 2300 Reservoir Avenue and the upgrade and renovations to Reservoir Avenue Pumping Station (which was adjacent to the pipes identified as being under capacity).

Noted.

6) In addition, since the review of 2300 Reservoir Avenue, 20 residential lots along Eddie Road, Rocky Hill Terrace and Garland Circle were identified for connection to the sewer system in this area (Contract 5). The sewer capacity request for this area has been accepted by the WPCA, therefore the estimated flows from these properties have been reserved in the existing conditions model, and have been calculated as follows:
   a. 20 lots, assuming an average of 2.8 residents per lot and 70 gpd/person = 3,920 gpd
   b. 1,550 LF of additional 8" sewer, assuming a 500 gpd/in-mile infiltration allowance = approx. 1,200 gpd
   c. Average Daily flow = 5,120 gpd for the average daily flow
   d. Peak flow = (3,920 * 5.6) + 1,200 = 23,152 gpd as a peak flow
   e. This flow is assumed to connect to the system through the 2300 Reservoir Development sewers, per the design agreement at that time, near Manhole 10B-75.

Noted. We understand that this project has not been completed yet and therefore the actual flows based on usage data for these residences is not available.

7) The Development at 2105 Reservoir was reviewed with a proposed sewer flow of 9,490 gpd average daily flow and 51,994 gpd at peak flow. Those flows are assumed to be 'existing flows' for the purposes of the capacity review of this development.

As noted above, the application of the 5.6x peaking factor based on the TR-16 design guide is extremely conservative and provides a potential peak flow estimate that is highly unlikely to occur. While the flow rates for this development were approved, empirical data would have provided a more accurate representation of the peak-flow conditions in the Reservoir Avenue sewer system.

We provide the following summary of our review of the available sewer capacity model downstream of the proposed development requesting connection to the system (2135, 2145 and 2155 Res Ave):

8) The sewers along Reservoir Avenue have sufficient capacity for the average daily flow, including this proposed development.
   a. The recent upgrade to Reservoir Avenue Pump Station provides adequate capacity for the average daily flows, through the station and its force main.
   b. The increased flow will mean that the pump station operates more frequently. Thus, piping downstream of the Reservoir Avenue pump station force main is unlikely to be affected.

Noted. We agree with this assessment.

9) The pipe segments along Reservoir Avenue with limited capacity are between SMH 10B-74 and 10B-76. These segments are 8" ACP sewers, with a slope of approximately 0.013 ft/ft, with a full pipe capacity of approximately 0.77 MGD.
   a. According to our capacity model, the existing flows including the proposed development at 2105 & 2125 Reservoir Avenue (as well as the Contract 5 service area along Eddie Road, Rocky Hill Terrace and Garland Circle), this segment would be required to convey a peak flow of 0.66 MGD, which places it at 85% full capacity at the peak flow conditions.
   b. With the addition of the peak flow rates from the proposed development from 2135, 2145 & 2155 Reservoir, these pipes would be required to convey a peak flow rate of approximately 0.81 MGD, more than their 0.77 MGD full pipe capacity.
      i. This equates to the segments being approximately 104.5% full or approximately 34,000 gpd over capacity.

      As noted above, based on a review of the calculations the existing model, including the 20 lots on Rocky Hill Terrace and the development at 2105 Reservoir Road, the average daily flow in the Reservoir Avenue system is 0.118 MGD, which is increased to 0.133 MGD when including the recalculated average daily flow for the Vessel development.

      Applying the corresponding peak factor for 0.133 MGD from Figure 2-1 in the TR-16, results in a peak flow of 0.692 MGD, which again is a potential peak flow in the Reservoir Avenue system for the maximum hour on the heaviest day of the year. Even based on the conservative factor applied throughout the model and the calculations, it appears that this segment of pipe would function under capacity (0.77 MGD) at peak flow conditions. *Under expected average flow conditions, the 0.133 MGD is only 17% of the 0.77 MGD full flow capacity of the pipe segment in question.*

A portion of the existing sewer system that would be used to convey wastewater from the proposed development does not have the capacity to convey the calculated peak flow. It is recommended to increase the pipes from MHs 10B-74 to 10B-76 to 10" PVC to provide the required capacity. This would leave one section of 8-inch pipe between two 10-inch pipes which is not standard sewer design practice. It is recommended to remove the 8-inch ACP from MH 10B-72A to MH 10B-74 and replace it with 10" PVC. The replacement of these 4 pipe segments consists of approximately 730 LF of sewer piping. This replacement would increase the capacity of the limiting pipe segments from 0.77 MGD to around 1.9 MGD. .

In summary, after reviewing the data presented in this memo and reevaluating the calculations, we have calculated that there is capacity in the sewer system to handle both the average daily flow of 14,560 gpd and the conservative peak flow of 73,950 gpd generated by the Vessel development. Guidance within TR-16 itself notes that empirical or actual flow data should be considered in any type of evaluation of a wastewater treatment facility. This is important when determining whether a real capacity issue exists or if the potential capacity issue is based on a theoretical model that may or may not be accurate.

If an actual capacity issue exists within the network, then there are also alternative solutions that can be utilized such as the installation of holding tanks on-site that can be fitted with pumps/valves to prohibit the release from effluent into the public sewer system until off-peak times. Two examples of concrete and HDPE holding tanks are attached for reference. An example of a commercial grinder pump is also attached for reference. Note that these are provided for reference only as a potential solution, and more detailed calculations are required prior to installation to confirm specific details of the equipment to be installed. If needed, on-site flow metering could also be conducted to ensure that effluent was only being released during non-peak periods.

We look forward to discussing our responses with you and with the WPCA. Please let us know if there are any further comments/questions or if any additional information is required to complete your review of this application.

TR-16

GUIDES FOR THE DESIGN OF WASTEWATER TREATMENT WORKS

2011 Edition

Prepared by the

New England Interstate Water Pollution Control Commission

650 Suffolk Street • Suite 410 • Lowell, MA 01854

Tel: 978-323-7929 • Fax: 978-323-7919

mail@neiwpcc.org • www.neiwpcc.org

Ronald F. Poltak, *Executive Director*

Compact Member States

Connecticut
Maine
Massachusetts
New Hampshire
New York
Rhode Island
Vermont



INTRODUCTION

The New England states and New York State are renowned for their lakes, streams, rivers, and coastal waters. These water resources are vital to the region's economy and are precious to its citizens. Effective wastewater treatment is absolutely fundamental to protecting these invaluable assets and to protecting public health. Thus, the proper design, construction, maintenance, and operation of wastewater treatment facilities is crucial. The region is fortunate to have many dedicated professionals who are committed to this effort and have been instrumental in the great successes that have been achieved.

For many years, these professionals have turned to the New England Interstate Water Pollution Control Commission's *Guides for the Design of Wastewater Treatment Works* as a helpful resource. Commonly known as TR-16 (short for Technical Report #16), this document provides guidance of value to engineers who are responsible for designing wastewater treatment plants, state regulators who are responsible for reviewing and approving the designs, and municipalities that may need assistance with the solicitation of professional design services for a wastewater facility. TR-16 is intended for use in conjunction with other available technical manuals, such as those produced by the Water Environment Federation (WEF).

The first edition of TR-16 was developed in 1962 in an effort to standardize wastewater treatment plants and to ensure consistency of design among the facilities in New England. Rapid advances in treatment technology necessitated the publication of an updated edition in 1980, and in 1998, amid the continued technological progress, NEIWPCC published the third edition of TR-16, which was developed with the assistance of a NEIWPCC workgroup formed specifically for the task.

In 2009, NEIWPCC's Executive Committee saw a need to revise the 1998 edition to incorporate advances in technology and eliminate out-of-date material. With guidance from the Executive Committee, NEIWPCC staff formed an advisory board made up of regional wastewater experts. The advisory board identified the changes needed in existing chapters and the new concepts that needed to be incorporated, such as new technologies, energy efficiency, and climate change implications. The board drafted an outline for the revised document, then recruited volunteers from the private and public sector to create the content. For each of the thirteen chapters, a writing group was formed, with one person taking on the key role of group chair. Under the supervision of NEIWPCC's project officer, the process of writing, reviewing, editing, and graphic design began, which ultimately resulted in the publication of this, the 2011 edition of TR-16.

The immense effort involved in creating this resource is justified when you consider what is at stake. Most wastewater treatment infrastructure is owned and operated by local municipalities and directly supported by taxpayers. This infrastructure is one of the largest assets of most communities. It is essential that it be designed to operate effectively and in a cost-effective manner. To achieve this goal, it is imperative that any design guidelines be as up-to-date as possible. Advances in wastewater treatment technology and energy efficiency, new approaches based on the need for sustainability—all must be incorporated, and all have been in this new edition. Note that the absence in this guide of design criteria for any specific process does not imply that the process is unacceptable.

This 2011 edition of TR-16 is the culmination of three years of writing, reviewing, editing, formatting, and coordinating. Its purpose, however, remains the same as in 1962, 1980, and 1998—to provide guidance in the design and preparation of plans and specifications for wastewater treatment works. As a guide, it is not to be construed as superseding the requirements, regulations, policies, and standards of the appropriate state water pollution control agencies. Users of this guide should be aware of all applicable regulations, and local and state regulators should always be contacted before starting facility planning. Users should also be familiar with federal requirements that apply to the development of engineering reports, plans, and specifications.

As is noted within this document, anticipated changes in weather patterns as a result of climate change and sea level rise should be factored into design. More precise predictions about these impacts are being developed and will change as more information is available; it is important to stay abreast of the latest thinking on this critical topic. Also, while this document focuses on traditional, centralized wastewater infrastructure, there is no denying that decentralized treatment systems can be a sound alternative. The New England states and New York State have developed separate guidance for decentralized systems, as well as for green infrastructure, stormwater, and other subjects of growing importance. To the extent possible, all these materials should be considered by municipalities seeking the best solutions for a clean environment.

Lastly, let us never forget that clean water is a finite resource that needs to be protected for future generations. Successful project designs should always have one priority above all others—the long-term protection of the precious water in our lakes, rivers, and streams.

1.2.4 Flow Rates

In assessing the flow rates for a particular collection and treatment system, include the following data, both existing and proposed, in the engineering report:

- Design period.
- Residential areas, including population density per acre.
- Area served in acres.
- Per capita wastewater average distribution. Calculate these flow rates using existing state data, information from technical journals, and/or actual water use rates.
- Infiltration rates that depend upon the year of construction, as well as type and size of collection sewers.
- Stormwater contribution.
- Industrial waste contribution.

Before submitting the engineering report, discuss the flow rates with the appropriate regulatory agency.

1.2.5 Loading Rates

Indicate in the engineering report the loading rates, or strength of wastewater, entering the wastewater treatment plant. State the biochemical oxygen demand, suspended solids, and total grease and oil for domestic wastewater and industrial waste. Make allowances for septage that commercial haulers may contribute to the system. As appropriate, list other wastewater constituents and characteristics, such as nitrogen, phosphorus, metals, organic chemicals, temperature, and chlorides. List average and maximum values for these constituents in the engineering report. Where new facilities are proposed, base loading rates on typical wastewater characteristics for the population to be served.

1.2.6 Initial and Final Operating Conditions

Since the quantity and quality of wastewater can vary from facility start-up to the end of the design period, state the initial and final operating conditions in the engineering report. To do so, the engineer must assess the planned development of the contributory area. This assessment may affect the future of the facility, either in terms of expansion of existing treatment units or abandonment of the facility location.

1.2.7 Seasonal Variations

When appropriate, the engineering report should include an assessment of seasonal variations in the quantity and quality of the wastewater, as well as the impact of these variations on the receiving water and treatment process facilities.

1.2.8 Process Schematic

The engineering report should include a process schematic showing the proposed wastewater collection and treatment processes. The process schematic should identify the process, size (including tank volume), necessary dimensions, and secondary flow processes. It should also identify secondary processes such as biosolids, treatment and disposal, chemical application, and compressed air.

1.2.9 Mass Balance

The engineering report should include a mass balance of the recommended wastewater treatment

CHAPTER 2

SANITARY SEWERS/ WASTEWATER COLLECTION SYSTEMS

Writing Chapter Chair:
Joseph M. Boccadoro, P.E., AECOM

Contributing Writers:
James Barsanti, P.E., Town of Framingham, MA
Virginia Roach, P.E., CDM

2.1 Design of Sanitary Sewers

This chapter provides guidance on the design and construction of sanitary sewers. Sanitary sewers include all gravity flow components of a wastewater collection system.

2.2 Design Capacity and Design Flow

2.2.1 Design Factors

In determining the required design parameters of sanitary sewers, the following factors must be considered:

- Peak hourly wastewater flow
- Additional peak flows of industrial and commercial wastes
- Maximum groundwater infiltration
- Topography of immediate area
- Difficulty of installation

2.2.2 Design Period

In general, wastewater collection systems should be designed for a life span of 50 years, and interceptor sewers should be designed to handle the maximum 50-year build-out tributary population. Communities should plan ahead for repair or replacement after 50 years of use. Consider the maximum capacity of uses such as institutions and industrial parks.

2.2.3 Design Flow

Submit to reviewing agency a detailed description of procedures used for sewer design.

2.2.3.1 Flow Related to Water Consumption

When available, use existing wastewater flow and/or water consumption data as a basis for sewer design. If such data are not available, consider using flow data from a similar

community or estimating flows based on the per capita usage described below or on the total number of bedrooms and square footage of non-residential uses served.

2.2.3.2 Per Capita Flow

Where actual flow data cannot be obtained, residential flows from new collection systems should be based on an average daily per capita flow of not less than 70 gallons per day (0.27 m^3/day).

2.2.3.3 Infiltration

Add an appropriate allowance to the capacity of sanitary sewers for infiltration due to normal aging of piping systems. An allowance of 250–500 gpd/in. diameter/mile of sewer (0.24–0.48 m^3/cm of pipe diam/km/day) is suggested as a normal range of infiltration. Amounts in excess of this estimate may be allowed if supporting documentation is provided. Evaluate soil and groundwater conditions in determining projected infiltration rates.

2.2.4 Peak Design Flow

Generally, sanitary sewers should be designed on a peak hourly design flow basis. The peak hourly flow rate is defined as the largest volume of flow to be received during a one-hour period and expressed as volume per unit time.

Wastewater flow consists of four components: domestic, commercial, industrial, and institutional. Where existing data are unavailable, peak domestic design flow may be determined using the ratio of peak to average daily flow as described in Figure 2-1 (from the Water Environment Federation/ American Society of Civil Engineers Manual of Practice (WEF/ASCE) publication, *MOP-FD-5 Gravity Sanitary Sewer Design and Construction*). Commercial, institutional, and industrial flows will generally have a different, lower peaking factor, depending on locations in a system and hours of operation.

When designing sewers to serve an existing sewered area, increased wet weather flow due to infiltration and inflow (I/I) must be considered. The area to be served should submit evidence that excessive I/I does not exist. If a reduction of I/I is proposed, a careful evaluation of the anticipated flow reduction should be made. Flow increases due to the elimination of sewer bypasses and backups should also be evaluated.

2.3 Details of Sewer Design and Construction

2.3.1 Minimum Size

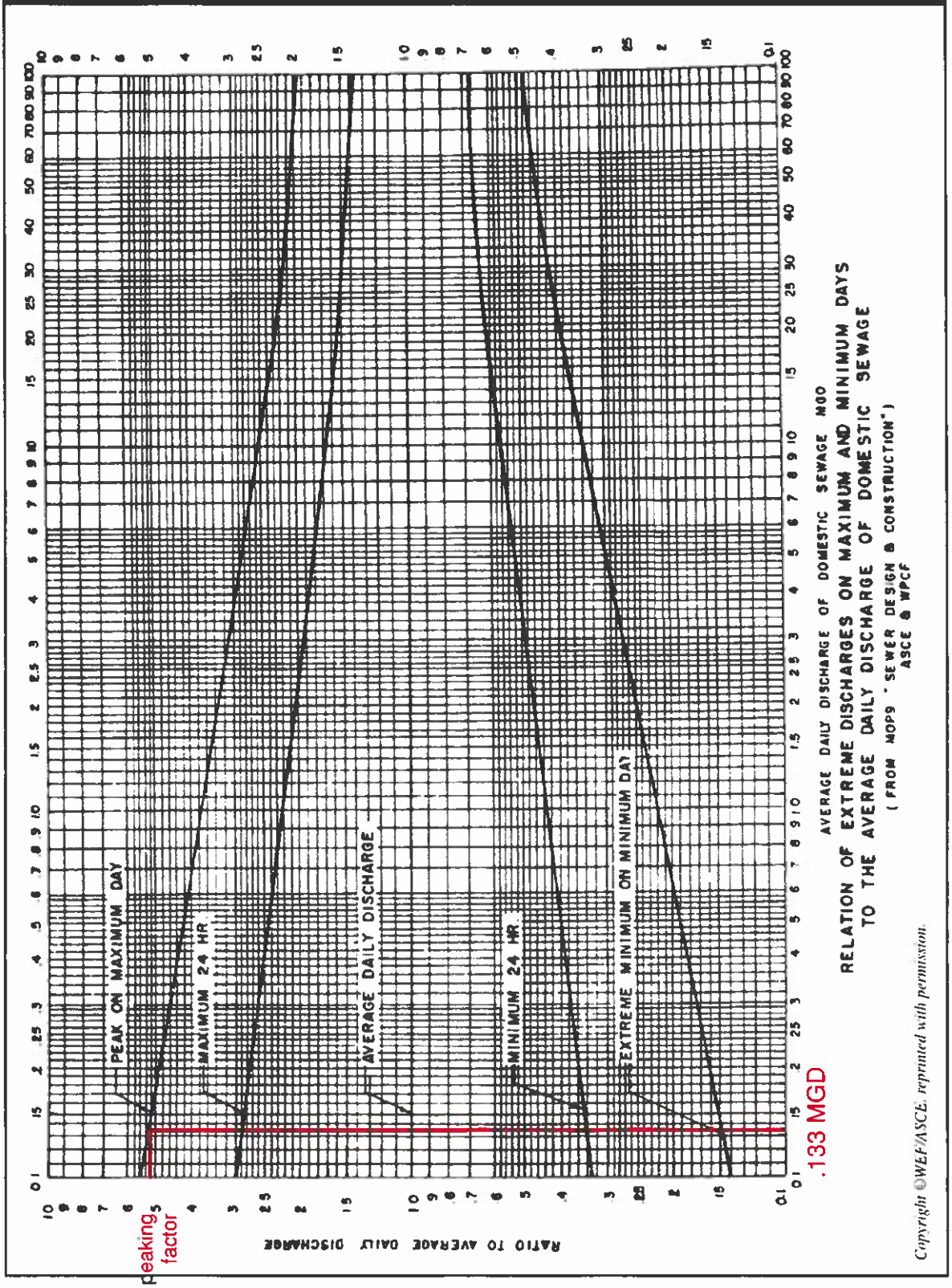
No public gravity sewer should be less than 8 inches in diameter (200 mm).

2.3.2 Depth

In general, sewers should be deep enough to drain basement fixtures and to prevent freezing. Provide insulation for sewers that cannot be placed deep enough to prevent freezing.

For house connections, chimneys (vertical pipe) should be considered when the sewer main is greater than or equal to 12 feet deep or the drop between the house connection and the main is greater than or equal to 3 feet. This should be considered only if the chimney is economical and meets minimum cover requirements for pipe protection. The design of chimneys should consider the vertical loads that will be exerted on the sewer main and fittings; an effective design transfers such loads from the pipe to the adjoining bedding and ground to prevent over-insertion, fracture, or misalignment.

Figure 2-1 **Ratio of Extreme Flow to Average Daily Flow**



New London Vessel

		AVG FLOWS
Units	30	32.86
Bedrooms	30	32.86
Occupants	34	29.00
Occupants / Bedroom	1.13	

Water Usage	Meter Read Dates		Usage (CF)	Usage (Gal)	Days	Average Daily Usage (Gal)	Average Per Occupant/Day
	2/27/2025	6/2/2025	14730	110188	95	1160	34
	6/2/2025	8/21/2025	11060	82735	80	1034	30
	8/21/2025	11/17/2025	8870	66352	88	754	22
Cumulative	2/27/2025	11/17/2025	34660	259275	263	986	29

Applied to Trumbull Vessel		**Average Flows / Unit**	**Average Daily Usage (Gal)**	**Altenate Recommended Calculation (gpd / occupant)**	**Recommended Total Average Daily Usage (Gal)**
Units	150				
Bedrooms	**178**	**32.86**	**5849**		
Occupants	**202**	**29.00**	**5857**	**70**	**14140**
Occupants / Bedroom	1.13				

City of New London
Department Of Public Utilities
15 Masonic Street
New London, CT 06320

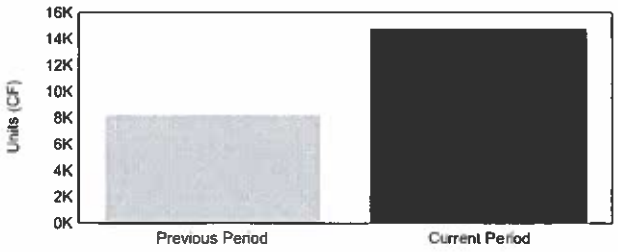
Phone: 860-447-5222 for customer service and after-hours emergencies

Customer - Account	Amount Due
061361 - 060286	$1,521.28
Due Date	**Delinquent Date**
06/05/2025	07/07/2025
174 BANK St	

1 CF = 1 unit of usage = 7.48 gallons

Meter #	Meter Size	Read Type	Read Dates Previous	Read Dates Current	Meter Readings Previous	Meter Readings Current	Usage (CF)	Days	Average Daily Usage (gallons)
70437816	3"	Actual Read	02/27/2025	06/02/2025	1014	2487	14,730	95	1,160

Your Quarterly Usage



MESSAGES

NEW PAYMENT LINK - http://payments.newlondonwwpca.org
To view the 2024 Consumer Confidence Report please follow this link: https://newlondonct.gov/water-quality-reports

On behalf of the City of New London and the Town of Waterford by VEOLIA

Account Summary

Previous Bills			$1,018.77
Payment - Thank You			-$1,018.77
Balance Forward			**$0.00**
Water			
New London Consumption	14,730 CF x	$0.02490	$366.78
NL Replacement Surcharge	14,730 CF x	$0.00420	$61.87
New London 3" Customer Facility			$69.76
Fire Service			
Fire Service - 4" - NL			$281.87
Sewer			
Consumption	14,230 CF x	$0.04750	$675.93
Base Charge			$9.56
Customer Facility Charge			$44.53
Stormwater			
Fee			$10.98
Total Current Charges			**$1,521.28**
TOTAL AMOUNT DUE:			**$1,521.28**

Please detach and return stub with payment.

City of New London
Department Of Public Utilities
15 Masonic Street
New London, CT 06320

Phone: 860-447-5222

Customer - Account	Amount Due
061361 - 060286	$1,521.28
Due Date	**Delinquent Date**
06/05/2025	07/07/2025

Total Amount Paid: _____

Service Address: 174 BANK St

00006136100006028600001521283

VRE CT001 174 BANK ST LLC
150 EUGENE ONIELL
NEW LONDON CT 06320

Please make checks Payable
CITY OF NEW LONDON
DEPARTMENT OF PUBLIC UTILITIES
PO BOX 4127
WOBURN MA 01888-4127

City of New London
Department Of Public Utilities
15 Masonic Street
New London, CT 06320

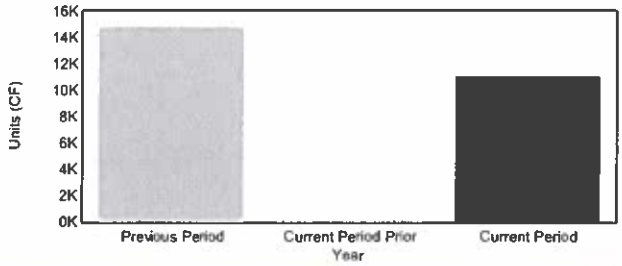
Phone: 860-447-5222 for customer service and after-hours emergencies

Customer - Account	Amount Due
061361 - 060286	$1,240.14
Due Date	**Delinquent Date**
09/05/2025	10/06/2025
174 BANK St	

1 CF = 1 unit of usage = 7.48 gallons

Meter #	Meter Size	Read Type	Read Dates		Meter Readings		Usage (CF)	Days	Average Daily Usage
			Previous	Current	Previous	Current			
70437816	3"	Actual Read	06/02/2025	08/21/2025	2487	3593	11,060	80	1,034

Your Quarterly Usage



MESSAGES

To view the 2024 Consumer Confidence Report please follow this link: https://newlondonct.gov/water-quality-reports
Please make checks payable to - City of New London.

On behalf of the City of New London and the Town of Waterford by VEOLIA

Account Summary

Previous Bills			$1,521.28
Payment - Thank You			-$1,521.28
Balance Forward			**$0.00**
Water			
New London Consumption	11,060 CF x	$0.02490	$275.39
NL Replacement Surcharge	11,060 CF x	$0.00420	$46.45
New London 3" Customer Facility			$69.76
Fire Service			
Fire Service - 4" - NL			$281.87
Sewer			
Consumption	10,560 CF x	$0.04750	$501.60
Base Charge			$9.56
Customer Facility Charge			$44.53
Stormwater			
Fee			$10.98
Total Current Charges			**$1,240.14**
TOTAL AMOUNT DUE:			**$1,240.14**

Please detach and return stub with payment.

City of New London
Department Of Public Utilities
15 Masonic Street
New London, CT 06320

Phone: 860-447-5222

Customer - Account	Amount Due
061361 - 060286	$1,240.14
Due Date	**Delinquent Date**
09/05/2025	10/06/2025

Total Amount Paid: _____

Service Address: 174 BANK St

00006136100006028600001240149

VRE CT001 174 BANK ST LLC
150 EUGENE ONIELL
NEW LONDON CT 06320

Please make checks Payable
CITY OF NEW LONDON
DEPARTMENT OF PUBLIC UTILITIES
PO BOX 4127
WOBURN MA 01888-4127

City of New London
Department Of Public Utilities
15 Masonic Street
New London, CT 06320

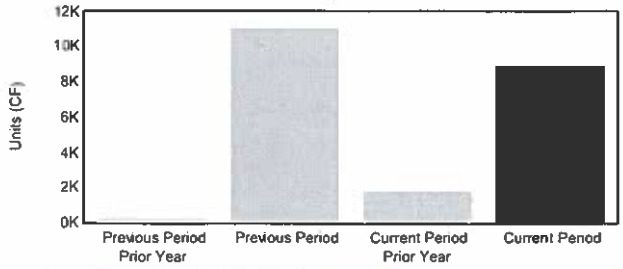
Phone: 860-447-5222 for customer service and after-hours emergencies

Customer - Account	Amount Due
061361 - 060286	$-99.90
Due Date	**Delinquent Date**
12/02/2025	01/02/2026
174 BANK St	

1 CF = 1 unit of usage = 7.48 gallons

Meter #	Meter Size	Read Type	Read Dates Previous	Read Dates Current	Meter Readings Previous	Meter Readings Current	Usage (CF)	Days	Average Daily Usage
70437816	3"	Estimated Read	08/21/2025	11/17/2025	3593	4480	8,870	88	754

Your Quarterly Usage



MESSAGES

To view the 2024 Consumer Confidence Report please follow this link: https://newlondonct.gov/water-quality-reports
NEW PAYMENT LINK - http://payments.newlondonwwpca.org
Please make checks payable to - City of New London.

On behalf of the City of New London and the Town of Waterford by VEOLIA

Account Summary

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Payment - Thank You			-$2,480.28
Balance Forward			**-$1,240.14**
Water			
New London Consumption	8,870 CF x	$0.02830	$251.02
NL Replacement Surcharge	8,870 CF x	$0.00480	$42.58
New London 3" Customer Facility			$79.52
Fire Service			
Fire Service - 4" - NL			$174.71
Sewer			
Consumption	8,370 CF x	$0.05700	$477.09
Base Charge			$11.48
Customer Facility Charge			$53.44
Stormwater			
Fee			$11.52

Other Charges	Charges
Interest - Water	$11.75
Interest - Sewer	$16.67
Interest - Fire Service	$8.46
Interest - Storm	$2.00
Total Current Charges	**$1,140.24**
TOTAL AMOUNT DUE:	**-$99.90**

Please detach and return stub with payment.

City of New London
Department Of Public Utilities
15 Masonic Street
New London, CT 06320

Phone: 860-447-5222

Customer - Account	Amount Due
061361 - 060286	$-99.90
Due Date	**Delinquent Date**
12/02/2025	01/02/2026

Service Address: 174 BANK St

00006136100006028600000000007

VRE CT001 174 BANK ST LLC
C / O KRC
342 North Main St
WEST HARTFORD CT 06117

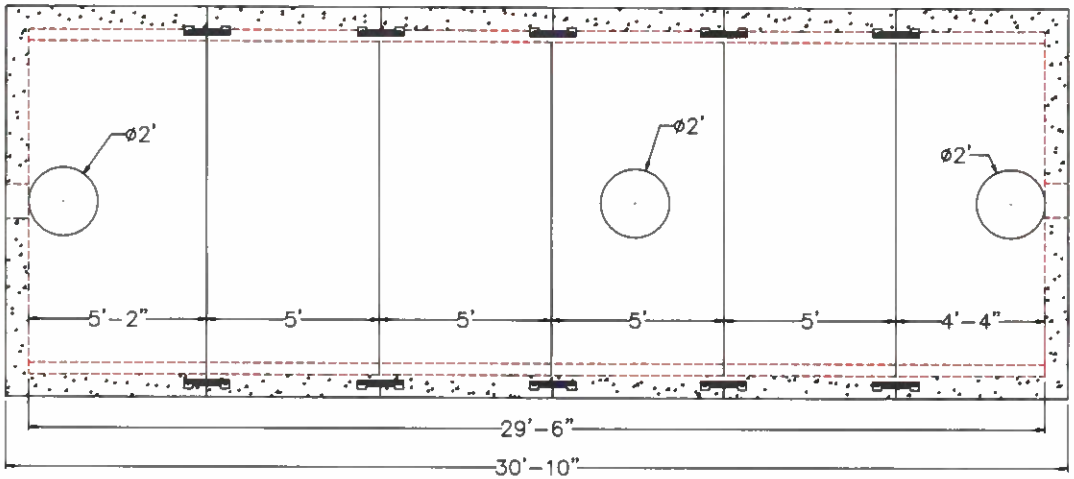
Please make checks Payable
CITY OF NEW LONDON
DEPARTMENT OF PUBLIC UTILITIES
PO BOX 4127
WOBURN MA 01888-4127

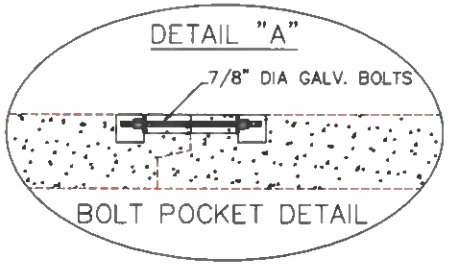
DESIGN NOTES:
1. CONRETE 5,000 PSI @ 28 DAYS
2. REINFORCING ASTM A-615 GRADE 60 #5 REBAR 12" O.C.B.W. OR EQUAL
3. STRUCTURE DESIGNED FOR H-20 LOADING WITH 18" TO 5' OF COVER
4. SEE CONDITIONS SHEET FOR PROPER SITE PREPARATION

SCITUATE COMPANIES
120 CLAY PIT ROAD
MARSHFIELD, MA 02050
PHONE # 1-800-440-0009
FAX # 781-837-4320

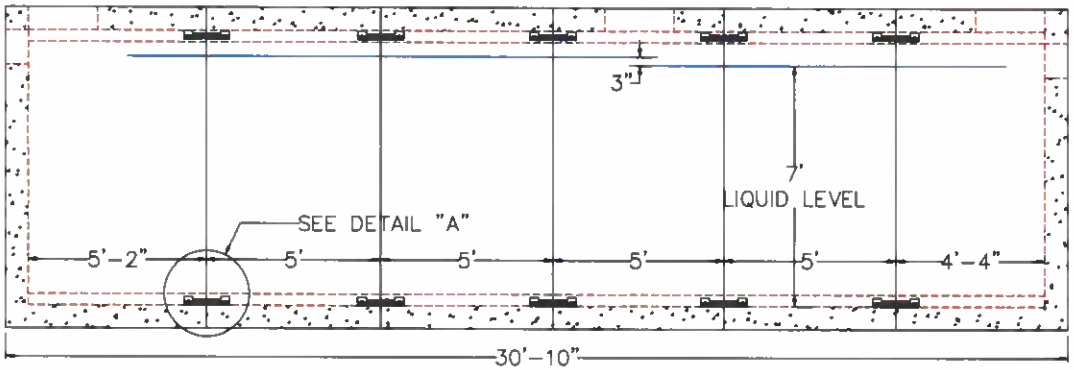
CONTRACTOR:	JOB NAME:	
DATE:	APPROVED BY:	DRAWING BY: C. J. SCOTT

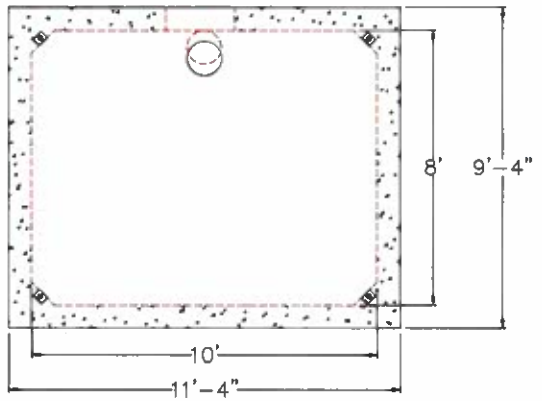
15,000 GALLON SINGLE COMPARTMENT H-20 TANK





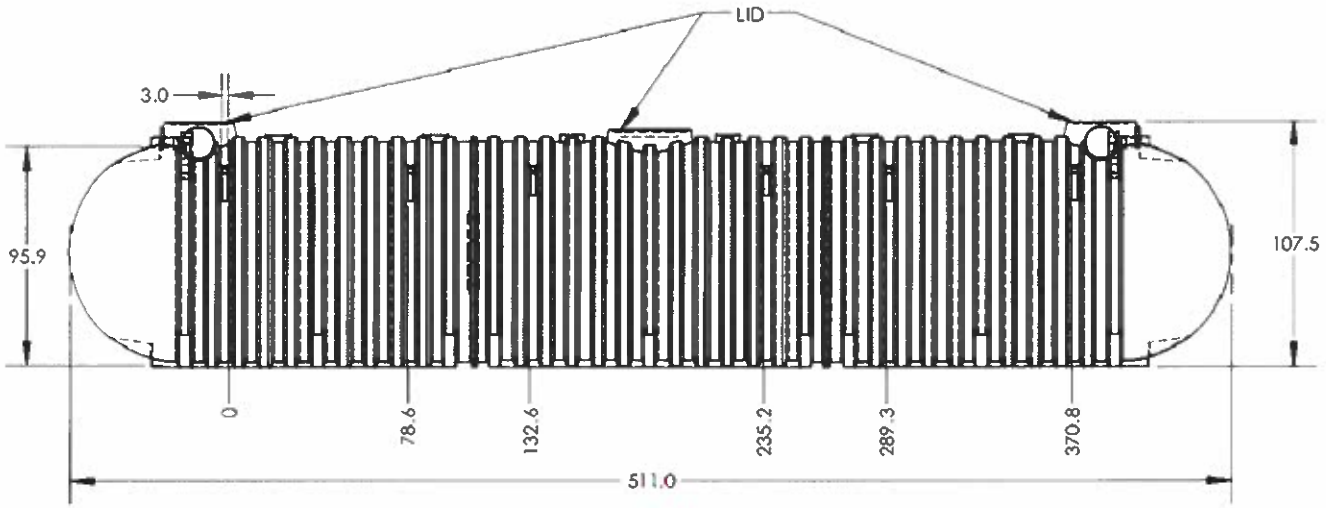
NPCA
Precast ... The Concrete Solution.

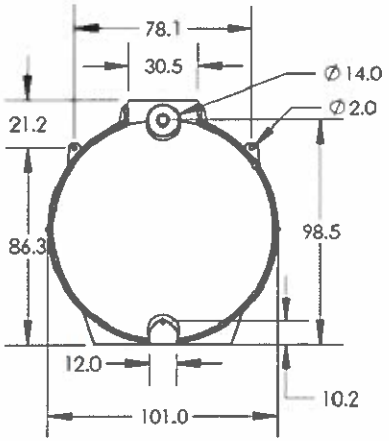




APPROVED FOR PRODUCTION BY _____ DATE _____

MATERIAL: HDPE
MAX TANK CAPACITY: 15336 GALLONS
CAPACITY AT 95.9": 15158 GALLONS
MAX BURIAL DEPTH: 24"





ALL DIMENSIONS ARE IN INCHES, NOMINAL, & SUBJECT TO CHANGE WITHOUT NOTICE.
ALL DIMENSIONS ON ROTATIONAL MOLDED PARTS ARE SUBJECT TO A ± 3% TOLERANCE.

DO NOT SCALE	DRAWN BY
STATUS: Released	TJK

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Bushman
A BRAND OF TANK HOLDING CORP.

TITLE	REVISION
15,000 Below Ground Assembly	B
	SHEET 1 OF 1
PART NO. 41615	ENG. ID. P020724

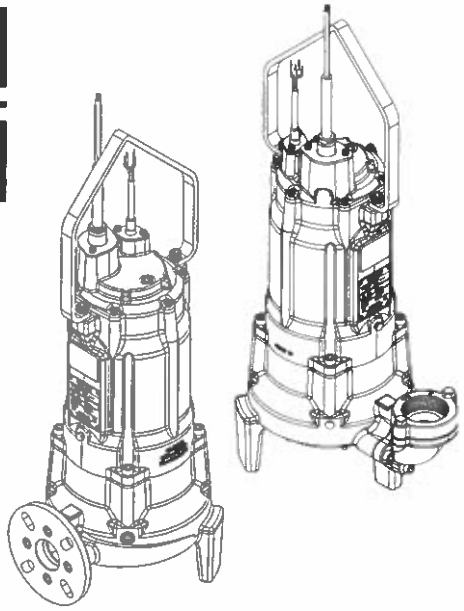
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A Family and Employee Owned Company

Omnivore® Grinders

Pump Specification

commercial series



LSGH300-Series

3 hp Grinder Pumps

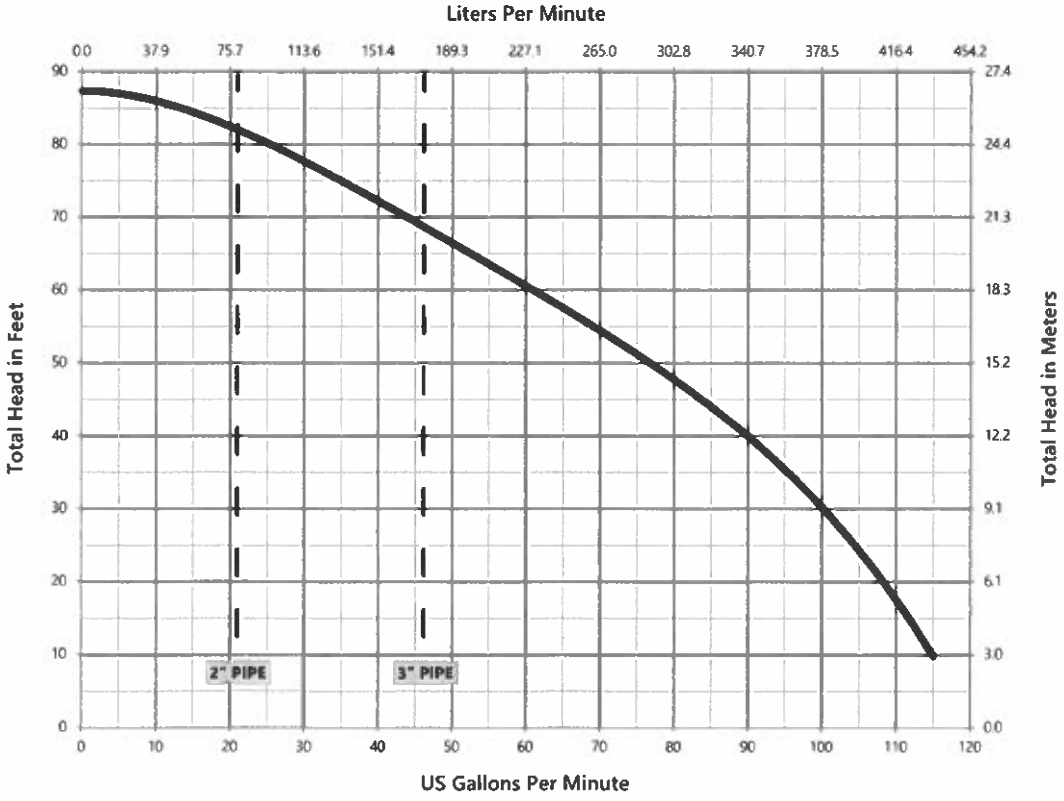
Horizontal Discharge, 2", 2.5" & 3" ANSI®

(DIN 50-PN10, DIN 65-PN10, DIN 80-PN6, RESPECTIVELY)

with 2" NPT

-or-

Vertical Discharge, 2"/3" NPT



ATTENTION

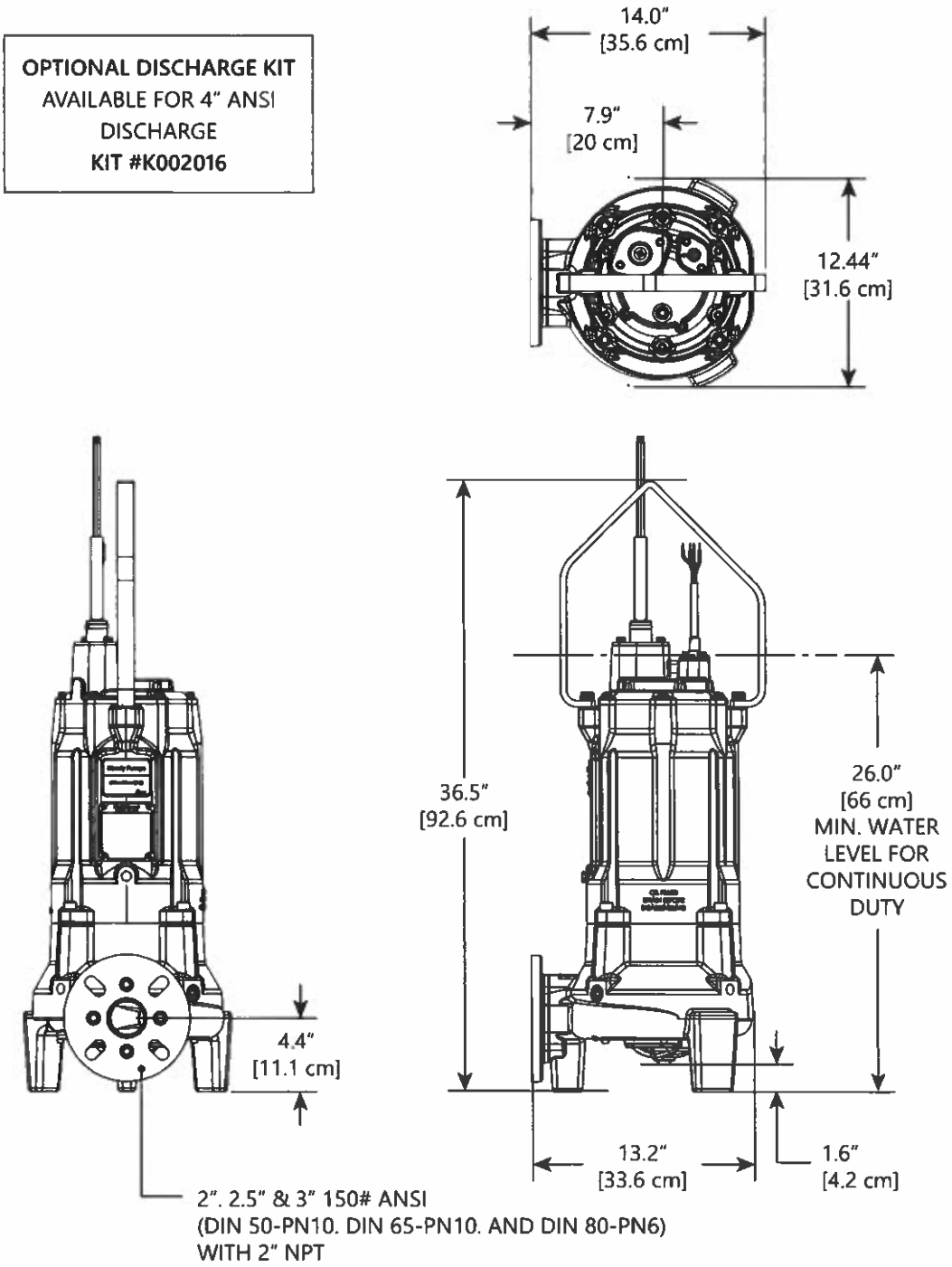
For pressure sewer applications, verify a *Redundant Check Valve Assembly* (curb stop and check valve) is installed between the pump discharge and the street main, as close to the public right-of-way as possible, on all installations to protect from system pressures.

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Horizontal Discharge

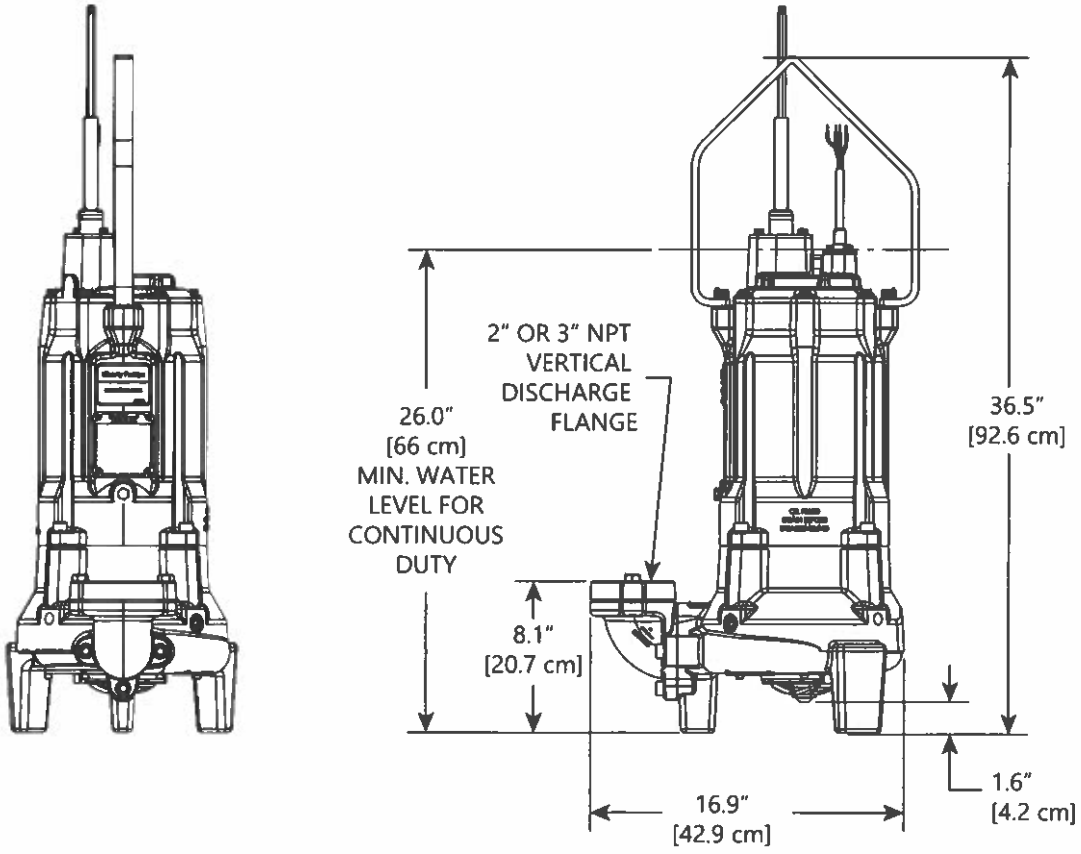
OPTIONAL DISCHARGE KIT
AVAILABLE FOR 4" ANSI
DISCHARGE
KIT #K002016



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Vertical Discharge



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LSGH300-Series Electrical Data

	MODEL[1]	HP	VOLTAGE	PHASE	FULL LOAD AMPS	LOCKED ROTOR AMPS	MAX KW INPUT	SERVICE FACTOR	POWER FACTOR	KVA CODE	CORD LENGTH
HORIZONTAL DISCHARGE	LSGH302M-3	3	230	1[2]	19	132	4.3	1	0.96	M	35'
	LSGH302M-5	3	230	1[2]	19	132	4.3	1	0.96	M	50'
	LSGH303M-3	3	200/230[3]	3	13/11.8	82	4.1	1	0.9	M	35'
	LSGH303M-5	3	200/230[3]	3	13/11.8	82	4.1	1	0.9	M	50'
	LSGH304M-3	3	460	3	5.8	41	4.1	1	0.88	M	35'
	LSGH304M-5	3	460	3	5.8	41	4.1	1	0.88	M	50'
	LSGH305M-3	3	575	3	5	33	4.3	1	0.88	M	35'
	LSGH305M-5	3	575	3	5	33	4.3	1	0.88	M	50'
VERTICAL DISCHARGE	LSGH302MV-3	3	230	1[2]	19	132	4.3	1	0.96	M	35'
	LSGH302MV-5	3	230	1[2]	19	132	4.3	1	0.96	M	50'
	LSGH303MV-3	3	200/230[3]	3	13/11.8	82	4.1	1	0.9	M	35'
	LSGH303MV-5	3	200/230[3]	3	13/11.8	82	4.1	1	0.9	M	50'
	LSGH304MV-3	3	460	3	5.8	41	4.1	1	0.88	M	35'
	LSGH304MV-5	3	460	3	5.8	41	4.1	1	0.88	M	50'
	LSGH305MV-3	3	575	3	5	33	4.3	1	0.88	M	35'
	LSGH305MV-5	3	575	3	5	33	4.3	1	0.88	M	50'

1. For 3-phase models operated on a variable frequency drive, VFD should be rated for a minimum of 1.5 times pump nameplate FLA.
2. Single-phase motors require capacitors and start switch. Liberty Pumps control panel or motor start circuit kits are recommended for proper operation and warranty. See ***Control Panel Information***.
3. System voltages: 208 and 240 volts with utilization voltages: 200 and 230 volts.

LSGH300-Series Control Panel Information

PUMP MODEL	VOLTAGE	3 FLOAT SIMPLEX	3 FLOAT DUPLEX	4 FLOAT DUPLEX
LSGH302	230	SXSKC24=3	AES24KC=3	AES24KC=4
LSGH303	200/230	SXS34=3-611	AES34=3-611	AES34=4-611
LSGH304	460	SXS34=3-171	AES34=3-171	AES34=4-171
LSGH305	575	SXS54=3-161	AES54=3-161	AES54=4-161

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LSGH300-Series Technical Data

MOTOR INSULATION CLASS	CLASS F 155°C / 311°F
IMPELLER TYPE / MATERIAL	SEMI-OPEN / STAINLESS STEEL
SHUT-OFF HEAD	88 FT
MIN HEAD	10 FT
MAX FLOW @ MIN HEAD	115 GPM
QUICK-CONNECT CORD	STANDARD
POWER CORD TYPE	SOOW
UPPER (INNER) SEAL MATERIAL	CARBON - ROTATING, CERAMIC - STATIONARY, VITON ELASTOMERS
LOWER (OUTER) SEAL MATERIAL	SILICON CARBIDE - ROTATING, GRAPHITE IMPREGNATED SILICON CARBIDE - STATIONARY, VITON ELASTOMERS
MAX WATER TEMP FOR CONTINUOUS DUTY	40°C / 104°F
MIN FLUID LEVEL FOR CONTINUOUS OPERATION	MOTOR HOUSING FULLY SUBMERGED
FLUID pH RANGE	4–10
STARTS PER HOUR	30
SHAFT MATERIAL	416 STAINLESS STEEL
FASTENER MATERIAL	316 STAINLESS STEEL
O-RING ELASTOMERS	BUNA-N
UPPER BEARING	SINGLE ROW DEEP GROOVE
LOWER BEARING	DOUBLE ROW ANGULAR CONTACT
OIL TYPE	ISO VG 10 TURBINE OIL
HORIZONTAL DISCHARGE	2", 2.5" & 3" 150# ANSI (DIN 50-PN10, DIN 65-PN10, DIN 80-PN6, RESPECTIVELY) WITH 2" NPT
VERTICAL DISCHARGE	2" & 3" NPT
PROTECTIVE EXTERNAL FINISH	POWDER COAT
SEAL FAIL DETECTION	DUAL PROBE - 2 WIRE WITH RESISTOR, 200K Ω RESISTANCE
THERMAL PROTECTION	HERMETICALLY SEALED THERMOSTATS, 125°C OPENING TEMPERATURE, 105°C CLOSING TEMPERATURE, 3A @ 120 VAC, 1A @ 240 VAC
VOLUTE MATERIAL	CLASS 30 CAST IRON
CUTTER MATERIAL	HARDENED 440 STAINLESS STEEL
CUTS PER MINUTE	OVER 400,000
APPROX WEIGHT	125 KG / 275 LBS
CERTIFICATIONS	CSA CERTIFIED TO CSA AND UL® STANDARDS, CAN/CSA C22.2 NO. 108-14, ANSI/UL 778

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LSGH300-Series Specifications

1.01 GENERAL

The contractor shall provide labor, material, equipment, and incidentals required to provide _____ (QTY) sewage grinder pumps as specified herein. The COMMERCIAL SERIES pump models covered in this specification are 1-phase model LSGH302, and 3-phase models LSGH303, LSGH304, and LSGH305 high head grinder pumps. The pump furnished for this application shall be model _____ as manufactured by Liberty Pumps.

2.01 OPERATING CONDITIONS

Each LSGH300-Series submersible pump shall be rated at 3 hp, _____ Volts, _____ phase, 60 Hz, 3450 RPM. The unit shall produce _____ GPM at _____ feet of total dynamic head.

The submersible pump shall be capable of handling residential and commercial sewage and grinding it to a fine slurry, enabling it to be pumped over long distances. The LSGH300-Series submersible pump shall have a shut-off head of 88 feet and a max flow of 115 GPM @ 10 feet of total dynamic head. The maximum submersion depth is 75 feet.

3.01 CONSTRUCTION

Each centrifugal sewage grinder pump shall be equal to the  Certified LSGH300-Series pumps as manufactured by Liberty Pumps, Bergen NY. The casting enclosing the motor shall be constructed of class 30 cast iron. The motor housing shall be oil-filled to dissipate heat. Air-filled motors shall not be considered equal since they do not properly dissipate heat from the motor. Mating parts shall be machined and sealed with a Buna-N O-ring. All fasteners exposed to the process fluid shall be stainless steel. The motor shall be protected on the top side with a sealed cast iron cord entry plate. The motor shall be protected on the lower side with a dual mechanical seal arrangement and an oil-filled intermediate chamber. The upper (inner) seal shall be a two-piece mechanical seal with a carbon rotating and a ceramic stationary face. The lower (outer) seal shall be a two-piece mechanical seal with a silicon carbide rotating and a graphite impregnated silicon carbide stationary face. The upper and lower bearings shall be sized to properly withstand radial and thrust loads produced throughout the full operating range of the pump.

4.01 POWER AND CONTROL CORD

The submersible pump shall be supplied with 35 or 50 feet of a multi-conductor cord of type SOOW. This type SOOW power cord carries a voltage rating of at least 600 V, a temperature rating of 90°C, have oil-resistant insulation, are water- and weather-resistant, UL listed, and CSA approved.

The power cord shall be sized for the rated full load amps of the pump for continuous duty in accordance with the NEC. A separate type SOOW control cord of equal length shall also exit the pump. Both the Quick-connect power and control cords shall have a molded plug held in place with a stainless steel compression plate. The plugs engage a receptacle with molded-in electrical pins, all of which shall be protected by a cast iron housing. The motor shall be protected from moisture via these two sealing methods: the plug to receptacle, and receptacle to cast iron housing. The Quick-connect cords offer quick cord replacement without the need to send the entire pump to an authorized repair facility and allows for pump maintenance without disturbing electrical boxes or control panels.

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5.01 MOTOR

The motor shall be oil-filled, inverter duty, Class F insulated, and rated for continuous duty. Since air-filled motors are not capable of dissipating heat efficiently, they shall not be considered equal. The mid chamber design utilizing **MidTherm™** Cooling technology shall allow for oil in the motor chamber to reject heat to the pumped media and provide cooling to the motor. This motor design shall provide significantly reduced operating temperatures. Pumps requiring an auxiliary cooling means shall not be considered equal.

The copper stator windings shall be insulated with moisture-resistant Class F insulation materials, rated for 155°C. The maximum continuous temperature of pumped liquids shall be 40°C. The winding operating temperature at rated horsepower and service factor shall be a maximum of 125°C @ 40°C ambient.

Motor shall have UL approved thermostats mounted directly on the stator windings. Motor shall have two thermostats on the 1-phase LSGH302 model, and three thermostats, one on each phase, on 3-phase models LSGH303, LSGH304, and LSGH305. The thermostat leads of the control cord shall be connected to a motor control relay in the control panel.

Motor service factor shall be 1.0 under normal conditions. Motor shall have a voltage tolerance of ±10% from nominal. Motor shall be capable of handling up to 30 evenly spaced starts per hour without overheating. Motor shall meet the requirements of NEMA MG1 Part 30 and 31 for operation on Pulse Width Modulation type VFD with inverter duty rated magnet wire and insulation. A VFD utilizing 3-phase input power shall be rated for a minimum of 1.5 times pump nameplate FLA. A VFD utilized to convert 1-phase input power to operate a 3-phase pump shall be rated a minimum of 2.0 times pump nameplate FLA.

6.01 BEARINGS AND SHAFT

The shaft shall be supported by two ball bearings. The top bearing shall be a deep groove radial contact ball bearing and the lower bearing shall be a double row angular contact ball bearing designed to handle the radial and axial forces incurred by pumping. The lower bearing shall be positively retained by a threaded bearing retaining nut, which eliminates any axial movement or rotation of the outer bearing race. Both bearings shall be permanently lubricated by the oil that fills the motor housing. Pump designs requiring scheduled bearing maintenance shall not be considered equal. Pumps with single row lower bearings or sleeve bearings shall not be considered equal. The bearing system shall be sized to provide a minimum of 100,000 hours B10 bearing life throughout the operating range of the pump. Pumps that only provide a 50,000 hour B10 bearing life shall not be considered equal.

The motor shaft extension shall be made of 416 stainless steel. The shaft shall be designed to withstand the maximum torque and radial loads present during start-up and normal operation.

7.01 SEALS

The pump shall have two shaft seals separated by an oil chamber. Pumps utilizing single seal technology shall not be considered equal. A leak detection probe shall be positioned in the oil chamber and shall allow for continuous monitoring for lower (outer) seal failure. The lower seal shall be a two-piece design that is easily serviceable. Shaft seals shall not require scheduled maintenance. The upper (inner) seal shall be carbon on ceramic and the lower seal shall be silicon carbide on graphite impregnated silicon carbide. Both seals shall include stainless steel housings and Viton elastomers.

8.01 IMPELLER

The impeller shall be stainless steel. The impeller shall be dynamically balanced, keyed and bolted onto the motor shaft.

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9.01 CONTROL PANEL

All LSGH300-Series pumps require a control panel. The control panel shall be equipped with circuit breakers and adjustable overload devices to protect against excess current or electrical problems. This device shall be sized appropriately for the pump model(s) being controlled.

The control panel shall include thermal overload relay(s) that shall shut down the pump in the event the thermostats in the motor open. Thermostats are designed to open at 125°C.

The control panel shall include a seal fail monitoring device that shall indicate a primary seal failure. The seal fail monitoring device shall be capable of monitoring the resistance of the seal failure probe in the pump. The sensitivity of the seal fail monitoring device shall be adjustable from approximately 1kΩ to at least 250kΩ. The LSGH300-Series was dual probe designed with a 200kΩ resistor so the circuit can be verified externally.

The control panel for 1-phase models shall include a motor start circuit that automatically engages and disengages the start windings of the 1-phase motor. A properly sized solid state motor start switch shall monitor start winding voltage and engage and disengage the start capacitor(s) from the circuit, as necessary. The start and run capacitors shall be properly sized per the motor specifications. A properly sized bleed resistor shall be used on the start capacitor(s) to avoid potentially damaging voltage spikes during startup. Capacitor start/run components and start switch (Kit #K002080) are available for 1-phase models using a non-Liberty Pumps control panel.

10.01 CUTTER MECHANISM

The cutter and plate shall consist of hardened 440 stainless steel with a Rockwell C hardness of 55-60. The stationary cutter plate shall have specially designed orifices through it, which enable the slurry to flow through the pump housing at an equalized pressure and velocity. The stationary cutter plate shall feature V-Slice® Cutter Technology. This superior cutting system consists of V shapes to maximize cutting action and arc shape exclusion slots to outwardly eject debris from under the rotary cutter. The rotary cutter shall have four blades and be designed with a recessed area behind the cutting edge to prevent the accumulation and binding of any material between rotary cutter and the stationary cutter plate. The cutter shall be capable of over 400,000 cuts per minute. The cutting system shall incorporate close tolerances for optimum performance. Ring or radial cutters, or those that grind on the outside circumference of shall not be considered equal.

11.01 PRESSURE SEWER APPLICATIONS

A redundant check valve assembly consisting of a curb stop and check valve must be installed between the pump discharge and the street main, as close to the public right-of-way as possible, on all pressure (force main) sewer installations to protect from system pressures. The curb stop valve is necessary to isolate the site from the pressure sewer while the check valve provides redundant protection against potentially detrimental backflow. All valves and fittings should be rated for at least 200 PSI service. See Liberty Pumps line of CSV-Series Curb Stop/Swing Check Valve Assemblies and CK-Series Connection Kit.

12.01 SHAFT GROUNDING BRUSH

3-phase pumps shall be equipped with a shaft grounding brush. Pumps not utilizing a current mitigation technology shall not be considered equal if operated on VFD.

13.01 QUALITY CONTROL

The pump shall be manufactured in an ISO® 9001 certified facility. Manufactured in the USA with US and global components.

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14.01 SUPPORT

The pump shall have cast iron support legs enabling it to be a freestanding unit. The legs shall be high enough to allow solids and long, stringy debris to enter the pump inlet.

15.01 PROTECTIVE FINISH

The exterior of the casting shall be protected with corrosion-resistant baked-on powder coat.

16.01 TESTING

The pump shall have a ground continuity check and undergo a hi-pot test for electrical integrity, moisture content, and insulation defects. The motor housing shall be pressurized, and an air leak decay test performed to ensure integrity of the assembled unit. The pump shall be run at rated voltage to verify current, performance curve, and monitor operation. Certified performance testing is available upon request.

17.01 WARRANTY

Liberty Pumps, Inc. warrants that pumps of its COMMERCIAL SERIES line are free from all factory defects in material and workmanship for a period of 18 months from the date of installation or 24 months from the date of manufacture, whichever occurs first, and provided that such products are used in compliance with their intended applications as set forth in the specifications and technical manuals. The date of installation shall be determined by a completed COMMERCIAL SERIES pump start-up report and warranty registration form.

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CARMODY
TORRANCE | SANDAK | HENNESSEY LLP

RECEIVED
FEB 25 2026
WATER POLLUTION CONTROL AUTHORITY
BY

Jason A. Klein
Partner
Phone: 203.252.2669
Fax: 203.325.8608
JKlein@carmodylaw.com

1055 Washington Blvd.
4th Floor
Stamford, CT 06901

February 10, 2026

VIA ELECTRONIC DELIVERY & FEDERAL EXPRESS

Town of Trumbull
Water Pollution Control Authority
366 Church Hill Road
Public Works Admin Building
Trumbull, CT 06611

Mr. William C. Maurer, PE, LS
Town Engineer
366 Church Hill Road
Public Works Admin Building
Trumbull, CT 06611
wmaurer@trumbull-ct.gov

James Nugent, Esq.
Harlow, Adams & Friedman, P.C.
One New Haven Ave., Milford, CT 06460
jmn@quidproquo.com

RE: 2135, 2145 and 2155 Reservoir Avenue, Trumbull, Connecticut (Assessor Map G 11, Lots 71, 73 and 74)
Sanitary Sewer Connection Request

Dear Commission Members, Mr. Mauer and Mr. Nugent:

Our firm represents Vessel Technologies, Inc., the parent company of Vessel RE Holdings, LLC (together hereinafter referred to as "Vessel" or the "Applicant"). I am writing to you regarding the Water Pollution Control Authority's (the "WPCA" or "Commission") consideration of Vessel's request to connect a proposed affordable housing development, submitted pursuant to Connecticut General Statutes § 8-30g ("8-30g" or the "Statute"), at 2135 – 2155 Reservoir Avenue, Trumbull, CT (the "Property") to the Trumbull sewer system (the "Proposal").

Both the Applicant's professional engineers and the Town Engineer have evaluated the system's performance under conservative, worst-case assumptions — including peak wet-weather flow conditions driven by extreme storm events and the potential for inflow and infiltration into the sanitary sewer. Using extremely conservative assumptions and modeling, Vessel's engineers have projected approximately 89% utilization of available pipe capacity for only one hour during the heaviest usage day/wettest day of the year.[1] The Town Engineer has more recently suggested a higher figure of approximately 97%, while noting that "the pipe segments can accept the flow from the proposed Vessel Development."[2] Both figures are projections based on conservative modeling. Critically, however, both analyses reach the same conclusion:

[1] Vessel's engineers maintain that this projection is likely higher than actual flows to be generated by the Proposal.
[2] A correspondence from William Mauer, Town Engineer, to Luke Mauro of Solli Engineering, dated February 3, 2026, is attached to this letter for your reference.

that the system retains capacity to accept the incremental flow associated with the Proposal under peak-case conditions.

A local water pollution control's decision to approve or deny a request to connect to an existing municipal sewer network must be based on substantial evidence within the record before it.[3] Speculative concerns do not amount to the substantial evidence necessary for a water pollution control authority to deny a connection request.[4] It is not entirely clear to the Applicant how the Town's professionals arrived at the conclusion that the pipe segment will be approximately 97% full during peak flow conditions. However, this number is a projection based on extremely conservative assumptions, and not on actual sewer usage data provided from another operational Vessel building within the State of Connecticut.

Vessel remains committed to providing the safe, affordable homes needed to address the housing crisis facing Connecticut. Approval of the Project will provide needed housing without overtaxing local infrastructure. Even so, Vessel is prepared to accept enforceable conditions enacted by the Commission to provide further assurances on this point. During the many months before any construction would occur, Vessel will undertake monitoring, sampling, and verification of actual capacity in the relevant pipe segment. If such testing demonstrates that the system would in fact be overburdened as a result of the Project, Vessel will implement those measures necessary to mitigate that impact, including, if required, the installation of on-site holding tanks and/or funding improvements within the public right-of-way. These measures may be in addition to the projected $1,050,000.00 connection fee for the proposed 150 apartments. Vessel will follow up within the next several days with additional details regarding these potential mitigation measures, including preliminary site planning for on-site storage.

Vessel respectfully requests a meeting in the coming days with the Commission's representatives, the Town Engineer, and relevant Town staff to discuss a clear process for conducting the proposed testing and based on the results, determining what remedial measures, if any, should be implemented as conditions of approval. Given the Commission will be considering the connection request in the near future, Vessel respectfully requests that this meeting take place within the following week. The Applicant believes this evidence-based pathway will fully address any remaining concerns while allowing this important affordable housing application to proceed and deliver needed homes to the people of Trumbull. Thank you for your time and attention regarding this matter.

Sincerely,

Jay Klein

Jason A. Klein

cc: J. Cordone james@cordonelaw.com
R. Librandi rlibrandi@trumbull-ct.gov

[3] See *Forest Walk LLC w. Water Pollution Control Authority*, 292 Conn. 271, 285 – 87 (2009).
[4] *See Armstrong v. Washington Zoning Board of Appeals*, 158 Conn. 158 (1969); *Milkus v. Fairfield Zoning Board of Appeals*, 154 Conn. 399 (1967).

From: William Maurer
To: Luke Mauro; Tatiana Solovey; christine.kurtz@wright-pierce.com
Cc: Alex Magoulas; Jason A. Klein; Roberto Librandi; Kyle Stein
Subject: RE: [EXTERNAL]RE: 2135-2155 Reservoir Avenue - Engineering & WPCA Comments
Date: Tuesday, February 3, 2026 7:52:39 AM
Attachments: image001.png

Hi Luke,

We are in receipt of your letter, dated January 20, 2026. responding to Wright-Pierce's review of the proposed Vessel Multifamily Development sewer connection on Reservoir Avenue.

In the time since our last meeting and before the receipt of this letter, Wright Pierce and the Town revisited our approach on the flow analysis and related pipe capacities. Adding the proposed flow (based on Wright-Pierce's determination) from the Vessel project to the flows already planned for the sewer segment being evaluated, the peaking factor from the Table in TR-16 can be revised. In lieu of the 5.6 stated in our review memo, we consider a peaking factor of 5.2 to be more appropriate for the updated flows. Based on this change, the pipe segments can accept the flow from the proposed Vessel Development; the pipe will be approximately 97% full during peak flow conditions.

I have forwarded your letter to Wright-Pierce; however, we do not intend to have them respond, nor do we intend to respond to the comments.

If you have any questions, please do not hesitate to contact me.

Bill

From: Luke Mauro <Luke@sollillc.com>
Sent: Wednesday, January 28, 2026 9:22 AM
To: Tatiana Solovey <tatianas@trumbull-ct.gov>; William Maurer <wmaurer@trumbull-ct.gov>; christine.kurtz@wright-pierce.com
Cc: Alex Magoulas <amagoulas@sollillc.com>; Jason A. Klein <jklein@carmodylaw.com>; Roberto Librandi <rlibrandi@trumbull-ct.gov>; Kyle Stein <kstein@myvessel.com>
Subject: RE: [EXTERNAL]RE: 2135-2155 Reservoir Avenue - Engineering & WPCA Comments

CAUTION: This email originated from outside your organization. Exercise caution when opening attachments or clicking links, especially from unknown senders.

Bill,

Just left you a voicemail. Following up to see if you and Christine have availability to discuss our responses to the WPCA comments and thoughts on the calculations we presented.

Thanks!

Luke Mauro, P.E., PTOE
Senior Project Manager

Solli

P: (203) 880-5455
C: (203) 927-2507
E: luke@sollillc.com
sollillc.com

From: Luke Mauro
Sent: Thursday, January 22, 2026 3:43 PM
To: 'Tatiana Solovey' <tatianas@trumbull-ct.gov>; 'William Maurer' <wmaurer@trumbull-ct.gov>; 'christine.kurtz@wright-pierce.com' <christine.kurtz@wright-pierce.com>
Cc: Alex Magoulas <amagoulas@sollillc.com>; 'Jason A. Klein' <jklein@carmodylaw.com>; 'Roberto Librandi' <rlibrandi@trumbull-ct.gov>; 'Kyle Stein' <kstein@myvessel.com>
Subject: RE: [EXTERNAL]RE: 2135-2155 Reservoir Avenue - Engineering & WPCA Comments
Importance: High

Hi Bill,

As discussed, please find attached our formal response to the WPCA comment letter from Wright-Pierce.

Please review and let us know if you and Christine have availability to discuss next week.

Thanks!

Luke Mauro, P.E., PTOE
Senior Project Manager

Solli

P: (203) 880-5455
C: (203) 927-2507
E: luke@sollillc.com
sollillc.com

From: Luke Mauro
Sent: Thursday, January 22, 2026 12:29 PM

Solli Engineering

Luke Mauro

Senior Project Engineer

RECEIVED
FEB 25 2026
WATER POLLUTION
CONTROL AUTHORITY
BY



Years With Firm

5-years

Certifications

Professional Engineer (P.E.)
Professional Traffic Operations Engineer (P.T.O.E.)

State Licensure

CT

Education

B.S. Civil Engineering; Rensselaer Polytechnic Institute

Experience

20-years of experience

Biography

Luke Mauro brings over 20 years of civil engineering experience, with a focus on site development, due diligence investigations, and transportation and traffic engineering. His expertise has supported a diverse portfolio of institutional, commercial, industrial, residential, and transportation projects for both public and private clients.

In land development, Luke has been responsible for pre-construction surveying, concept planning, detailed site design, stormwater management, utility design and coordination, construction cost estimating, and comprehensive due diligence investigations. His proficiency extends to zoning code reviews, real estate closing support, and the preparation of permitting and construction documents, ensuring seamless project execution.

Luke's traffic engineering expertise includes the preparation of traffic impact studies, parking studies, traffic signal design, and conceptual and detailed roadway design. He also excels in the development of maintenance and protection of traffic plans, as well as permitting coordination with local and state agencies, including the Connecticut Department of Transportation (CTDOT) District Offices and the Office of the State Traffic Administration (OSTA).

With his breadth of experience and technical acumen, Luke plays a pivotal role in delivering innovative and practical engineering solutions tailored to client needs.

Representative Projects



Quarry Walk, Oxford, CT – The transformation of a former quarry on Route 67 into a premier mixed-use development included commercial spaces, multifamily residential units, and extensive recreational and open space areas. As Project Manager, Luke Mauro oversaw site design, traffic engineering, and permitting services, ensuring a seamless integration of infrastructure and site operations.

Representative Projects (Cont)...

Luke's site design responsibilities encompassed conceptual and master planning, detailed site design, grading, drainage, erosion, and sediment control. He also managed utility design and provided permitting support with local agencies and the Connecticut Department of Energy and Environmental Protection (CTDEEP). For traffic engineering, Luke prepared detailed off-site improvement plans and traffic control signal plans, conducted intersection operations analysis using Synchro software, and developed a traffic impact study. He performed signal warrant analyses and coordinated with the Office of the State Traffic Administration (OSTA) and the Connecticut Department of Transportation (CTDOT) District 4 for MTG and encroachment permits.

The project design included new sidewalks along the Oxford Road frontage and a multi-use path around the property perimeter, creatively located within a two-tiered retaining wall to maximize space and functionality.



The SoNo Collection, Norwalk, CT* – The project involved the construction of a ±750,000-square-foot high-end retail shopping mall on West Avenue in Norwalk, strategically located at the intersection of Interstate 95 and Route 7. Luke Mauro served as the Project Manager, overseeing traffic engineering and permitting services critical to the project's success. His traffic engineering responsibilities included preparing conceptual and detailed off-site improvement plans, multiple traffic control signal plans, maintenance and protection of traffic plans, detour plans, and intersection operations analyses using Synchro software.

Mr. Mauro also developed a comprehensive traffic impact study, conducted signal warrant analyses, and managed extensive coordination with the Office of the State Traffic Administration (OSTA), the Connecticut Department of Transportation (CTDOT) District 3, and the City of Norwalk. His efforts included MTG permitting, encroachment permitting, local roadway permitting, and support during construction, ensuring that the project's traffic infrastructure seamlessly integrated with the surrounding road network.



Amazon Fulfillment Center, North Haven, CT – The project involved the construction of a ±850,000-square-foot Amazon Fulfillment Center on Washington Avenue (U.S. Route 5) in North Haven. Luke Mauro served as the Project Manager, overseeing the traffic engineering and permitting services essential to the project's development. His responsibilities included preparing conceptual and detailed off-site improvement plans, multiple traffic control signal plans, and maintenance and protection of traffic plans. He also conducted intersection operations analyses using Synchro software and developed a comprehensive traffic impact study.

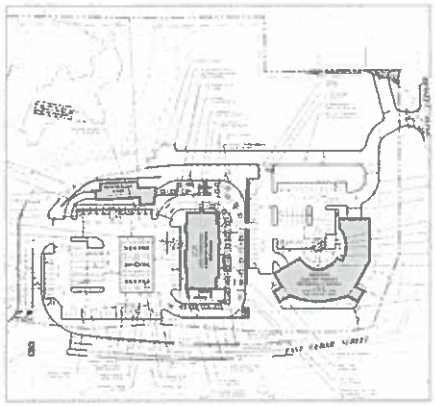
Representative Projects (Cont)...

Mr. Mauro coordinated extensively with the Office of the State Traffic Administration (OSTA) and the Connecticut Department of Transportation (CTDOT) District 3. His efforts encompassed MTG permitting, encroachment permitting, and providing construction support, ensuring the successful integration of the fulfillment center's traffic infrastructure with the surrounding transportation network.



Noble Energy Development, Enfield, CT – The project involves the development of an 8,384-square-foot convenience store with a drive-thru and a 3,398-square-foot car wash located at the intersection of Elm Street and Shaker Road in Enfield. Luke Mauro served as the Senior Project Manager, overseeing site design, traffic engineering, and local and state permitting for both on-site and off-site work associated with the development.

His traffic engineering responsibilities included preparing a comprehensive traffic impact study, developing off-site improvement plans, and designing traffic signals. Mr. Mauro also managed permitting coordination with the Connecticut Department of Transportation (CTDOT) District 1 and the Town of Enfield, ensuring smooth integration of the development into the local transportation network.



Noble Energy Development, Newington, CT – The project involves the development of an 8,384-square-foot convenience store with a drive-thru, a 3,398-square-foot car wash, and a 23,500-square-foot electric vehicle showroom and service center located at the intersection of East Cedar and Russell Road in Newington. Luke Mauro served as the Senior Project Manager, overseeing the site design, traffic engineering, and local and state permitting for both on-site and off-site components of the development.

His traffic engineering responsibilities included preparing a comprehensive traffic impact study, designing off-site improvements and traffic signals, and managing highway design. Mr. Mauro also coordinated permitting efforts with the Connecticut Department of Transportation (CTDOT) District 1 and the Town of Newington, ensuring the project adhered to all necessary requirements and integrated seamlessly with the surrounding infrastructure.

**Indicates project done while employed by another firm.*



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PROPOSED DEVELOPMENT

2135, 2145, 2155 RESERVOIR AVE
TRUMBULL, CONNECTICUT

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BY _____

Vessel


N
Reservoir
Pump Station
10B-97
10B-98
10B-95
10B-96
10B-2
10B-4
10B-5
10B-1
10B-3
10B-61
10B-73
10B-72A
10B-73
10B-74
10B-H1A
10B-H3
10B-75
10B-76
10B-77
10B-88
10B-87
10B-86
10B-85
10B-84
10B-78
10B-79
10B-80
10B-82
10B-93
10B-94
10B-92

Existing/Background Flow Calculations

Approximate Existing Flow in System

- Average Flow = 0.105 MGD or 105,000 gpd
- Infiltration & Inflow (I/I) ≈ 0.01-0.02 MGD (negligible 1-2% of pipe capacity)
- Capacity of Sewer Pipe Used ≈ 0.105 / 0.77 = 13.6%

20 Lots on Rocky Hill Terrace, Eddie Road & Garland Circle

- 2.8 occupants/lot x 70 gpd/person = 196 gpd/lot
- 2.8 occupants/lot ÷ 3.2 bedrooms/lot = 0.875 occupants/bedroom
- 196 gpd/lot ÷ 3.2 bedrooms/lot = 61.25 gpd/bedroom
- Approved Total Flow = 3920 gpd + 1,200 I/I = 5,120 average gpd

2105-2125 Reservoir Avenue

- Approved Total Flow = 9,490 average gpd

Approximate Background Flow in System

- Average Daily Flow = 0.119 MGD or 119,000 gpd
- Capacity of Sewer Pipe Used ≈ 0.119 / 0.77 = 15.5%

- Typical Daily Peak Flow (3x) ≈ 0.337 MGD or 337,000 gpd
- Capacity of Sewer Pipe Used under Peak ≈ 0.337 / 0.77 = 43.8%

- Peak on Max Day (5.2x per TR-16) ≈ 0.577 MGD or 577,000 gpd
- Capacity of Sewer Pipe Used under Peak ≈ 0.577 / 0.77 = 74.9%

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Vessel New London Empirical Usage Data

New London Vessel

		AVG FLOWS
Units	30	32.86
Bedrooms	30	32.86
Occupants	34	29.00
Occupants / Bedroom	1.13	

Water Usage	Meter Read Dates		Usage (CF)	Usage (Gal)	Days	Average Daily Usage (Gal)	Average Per Occupant/Day
	2/27/2025	6/2/2025	14730	110188	95	1160	34
	6/2/2025	8/21/2025	11060	82735	80	1034	30
	8/21/2025	11/17/2025	8870	66352	88	754	22
Cumulative	2/27/2025	11/17/2025	34660	259275	263	986	29

Applied to Trumbull Vessel		**Average Flows / Unit**	**Average Daily Usage (Gal)**	**Altenate Recommended Calculation (gpd / occupant)**	**Recommended Total Average Daily Usage (Gal)**
Units	150				
Bedrooms	**178**	**32.86**	**5849**		
Occupants	**202**	**29.00**	**5857**	**70**	**14140**
Occupants / Bedroom	1.13				

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Vessel Trumbull Estimated Sewer Usage

Based on Empirical Data

Average Rate = 5,850 gpd + 420 gpd (I/I) = 6,270 gpd
Peak Rate (3x) on Typical Day from TR-16 = 17,970 gpd (over an hour)

Based on TR-16 Methodology

Average Rate = 14,140 gpd + 420 gpd (I/I) = 14,560 gpd
Peak Rate (3x) on Typical Day from TR-16 = 42,840 gpd (over an hour)

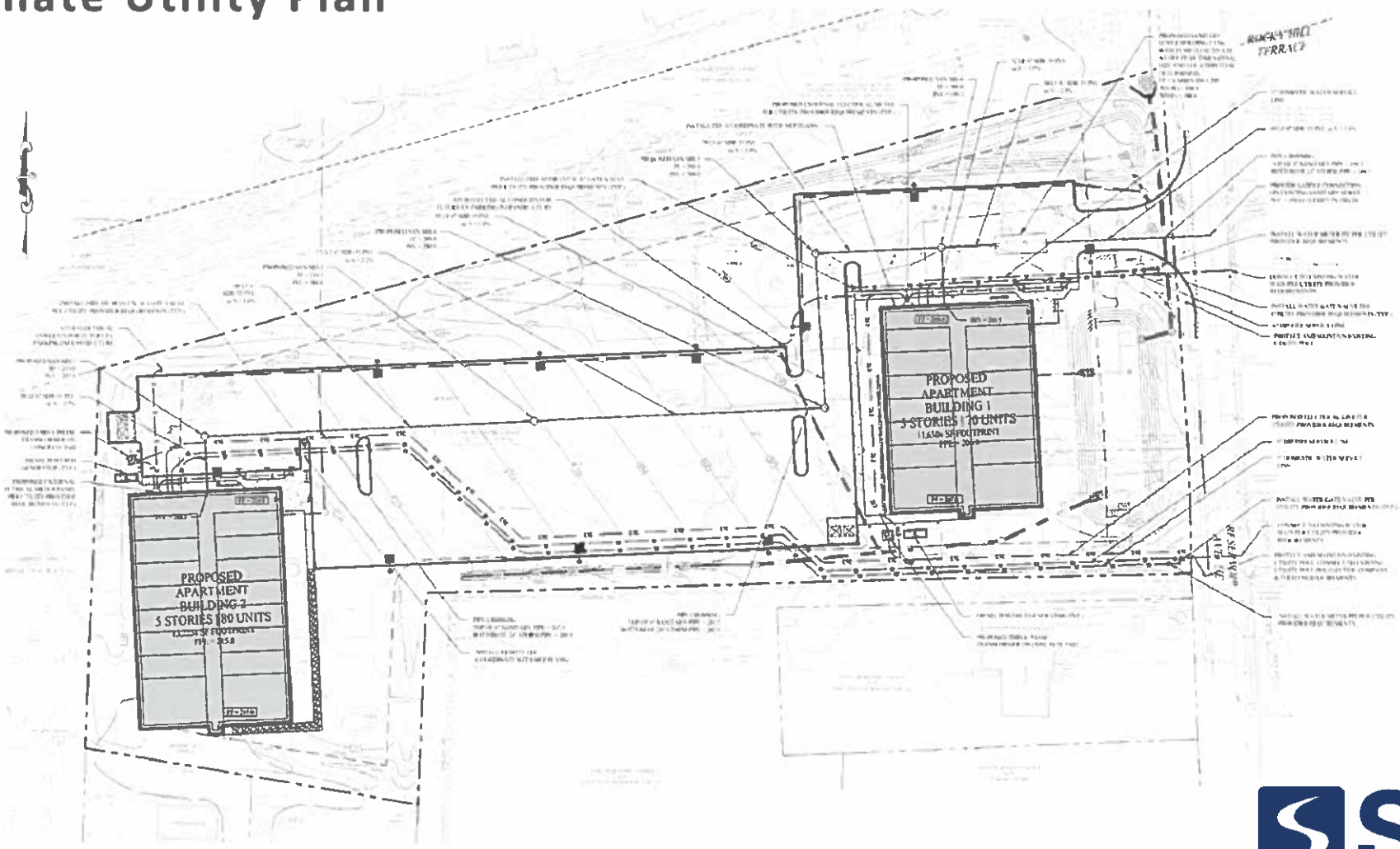
Approximate Future Flow in Reservoir Avenue System

Average Daily Flow (Empirical) ≈ 0.125 MGD or 125,000 gpd ≈ 16.2% capacity
Peak Daily Flow (Empirical) ≈ 0.355 MGD or 355,000 gpd ≈ 46.1% capacity
Peak on Max Day (5.2x per TR-16) ≈ 0.610 MGD or 610,000 gpd ≈ 79.2% capacity

Average Daily Flow (TR-16) ≈ 0.133 MGD or 133,000 gpd ≈ 17.3% capacity
Peak Daily Flow (TR-16) ≈ 0.380 MGD or 380,000 gpd ≈ 49.4% capacity
Peak on Max Day (5.2x per TR-16) ≈ 0.650 MGD or 650,000 gpd ≈ 84.4% capacity

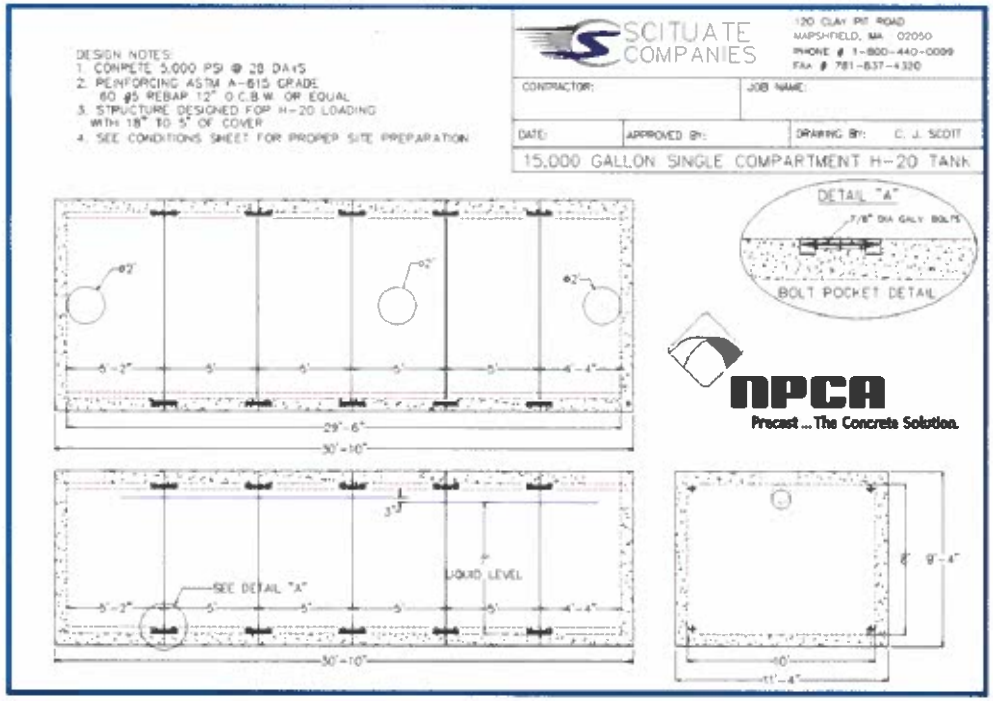
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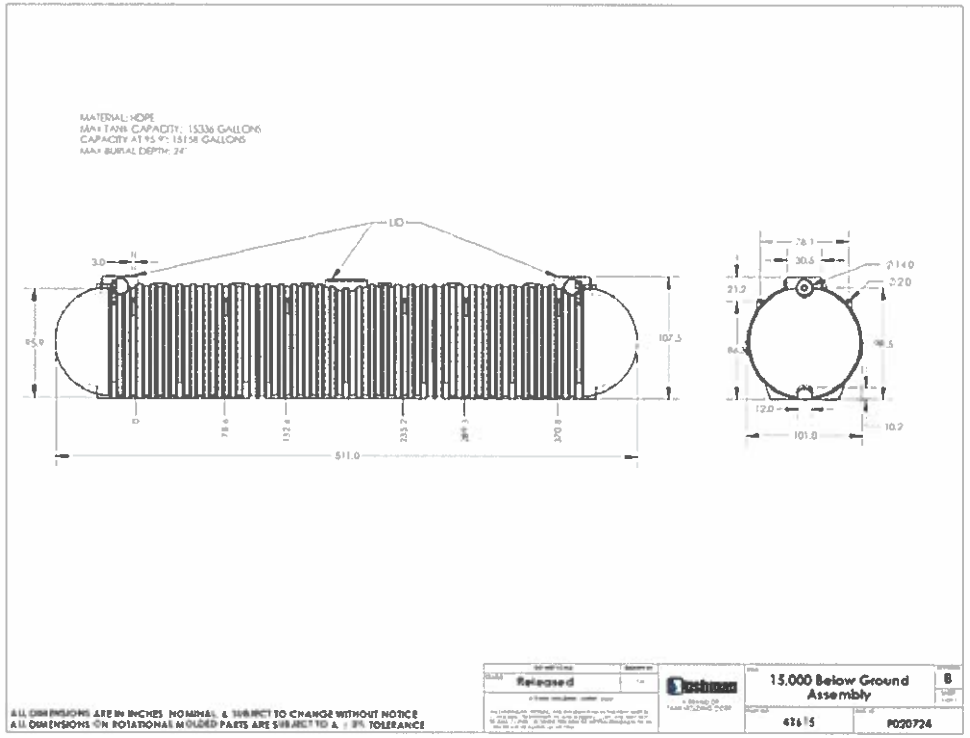
Alternate Utility Plan



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Example Holding Tank & Pump Details





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Example Holding Tank & Pump Details

Liberty Pumps
A Family and Employee Owned Company

Omnivore

Pump Specification

commercialseries

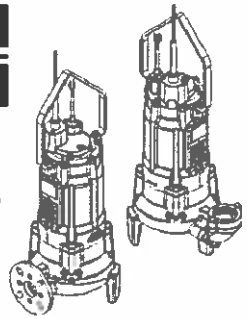
LSGH300-Series

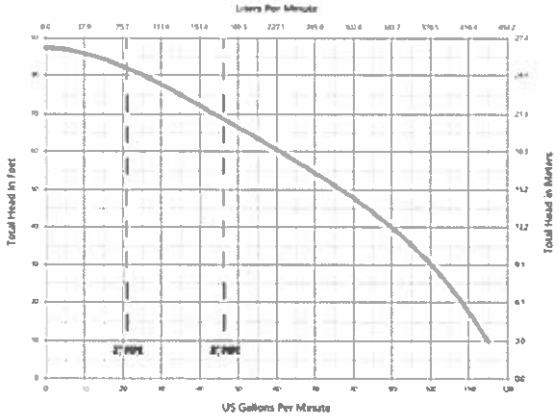
3 hp Grinder Pumps

Horizontal Discharge, 2", 2.5" & 3" ANSI®
(DIN 50-PN10, DIN 65-PN10, DIN 80-PN6, RESPECTIVELY)
with 2" NPT

-or-

Vertical Discharge, 2"/3" NPT





ATTENTION

For pressure sewer **applications**, verify a ***Redundant Check Valve Assembly*** (curb stop and check valve) is installed between the pump discharge and the street main, as close to the public right-of-way as possible, on all installations to protect from system pressures.

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LSGH300 P1 R05/2025

7000 Apple Tree Avenue Bergen NY 14416 ● Phone 1-800 543-7550 ● Fax 1-585-494-1870 ● Email Liberty@LibertyPumps.com ● Web www.LibertyPumps.com

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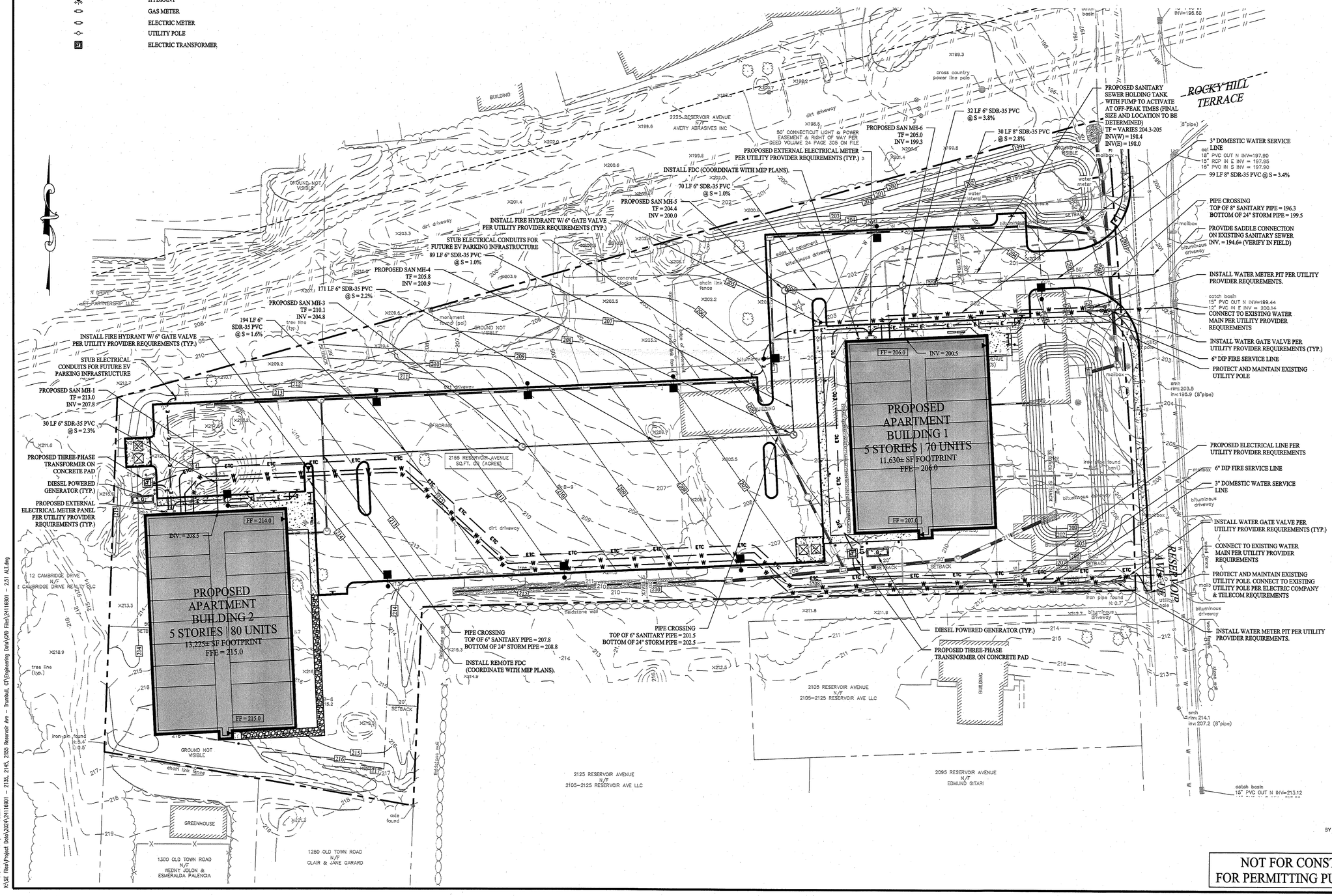
PROPOSED DEVELOPMENT

2135, 2145, 2155 RESERVOIR AVE
TRUMBULL, CONNECTICUT

Vessel

LEGEND

- PROPERTY LINE
- RIGHT-OF-WAY LINE
- ADJOINING LOT LINE
- W — W — W — WATER MAIN / LATERAL
- G — G — G — GAS LINE
- E — E — E — ELECTRIC CONDUIT
- CTV — CABLE & TELEPHONE CONDUIT
- ETC — UNDERGROUND ELECTRIC, TELEPHONE AND CABLE LINES
- SANITARY SEWER PIPE
- SANITARY SEWER MANHOLE
- WATER VALVE
- HYDRANT
- GAS METER
- ELECTRIC METER
- UTILITY POLE
- ELECTRIC TRANSFORMER



GENERAL NOTES

1. THESE PLANS ARE FOR PERMITTING PURPOSES ONLY AND ARE NOT FOR CONSTRUCTION. NO CONSTRUCTION OR DEMOLITION SHALL BEGIN UNTIL FINAL APPROVAL OF THIS PLAN IS GRANTED BY ALL GOVERNING AND REGULATORY AGENCIES.
2. ALL PROPOSED SITE WORK IS TO BE COMPLETED IN ACCORDANCE WITH ALL PERMITS, APPROVALS, AND CONDITIONS OF APPROVALS ISSUED BY LOCAL, STATE AND/OR FEDERAL REVIEWING AGENCIES.
3. EXISTING BOUNDARY AND TOPOGRAPHY IS BASED ON DRAWING TITLED "PROPERTY SURVEY OF 2135, 2145 & 2155 RESERVOIR AVENUE, TRUMBULL, CONNECTICUT, PREPARED FOR VESSEL TECHNOLOGIES", SCALE 1"=30', DATED JULY 24, 2025, PERFORMED BY ACCURATE LAND SURVEYING, LLC.
4. ALL CONSTRUCTION SHALL COMPLY WITH TOWN OF TRUMBULL STANDARDS, CONNECTICUT DEPARTMENT OF TRANSPORTATION STANDARDS AND SPECIFICATIONS IN THE ABOVE REFERENCED INCREASING HIERARCHY. IF SPECIFICATIONS ARE IN CONFLICT, THE MORE STRINGENT SPECIFICATION SHALL APPLY. ALL CONSTRUCTION SHALL BE PERFORMED IN ACCORDANCE WITH ALL APPLICABLE OSHA, FEDERAL, STATE AND LOCAL REGULATIONS.

SITE UTILITY NOTES

1. SEE SHEET 2.21 FOR DETAILS REGARDING THE PROPOSED GRADING AND DRAINAGE FEATURES, STORMWATER CONVEYANCE, AND STORMWATER DETENTION FACILITIES.
2. THE CONTRACTOR SHALL FIELD VERIFY ALL EXISTING CONDITIONS, UTILITY LOCATIONS, AND INVERTS PRIOR TO CONSTRUCTION. ANY CONDITIONS FOUND TO DIFFER FROM THOSE SHOWN IN THE DRAWINGS SHALL BE IMMEDIATELY BROUGHT TO THE ATTENTION OF THE SITE/CIVIL ENGINEER.
3. CONTRACTOR SHALL REFER TO ARCHITECTURAL PLANS AND SPECIFICATIONS FOR ACTUAL LOCATIONS OF ALL UTILITY ENTRANCES TO INCLUDE SANITARY SEWER LATERALS, DOMESTIC AND FIRE PROTECTION WATER SERVICE, ELECTRICAL, TELEPHONE AND GAS SERVICE, ROOF DRAINS, AND ALL OTHER UTILITIES. CONTRACTOR SHALL COORDINATE INSTALLATION OF UTILITIES IN SUCH A MANNER AS TO AVOID CONFLICTS AND TO ENSURE PROPER DEPTHS ARE ACHIEVED AS WELL AS COORDINATING WITH THE REGULATORY AGENCY AS TO LOCATION OF AND SCHEDULING OF CONNECTIONS TO THEIR FACILITIES.
4. THE CONTRACTOR SHALL BE RESPONSIBLE FOR NOTIFYING UTILITY COMPANIES 72 HOURS PRIOR TO BEGINNING EXCAVATION AND INSTALLATION.
5. THE CONTRACTOR SHALL BE RESPONSIBLE FOR OBTAINING ALL NECESSARY PERMITS AND ALL PAVEMENT REPAIRS REQUIRED AS A RESULT OF ANY UTILITY WORK.
6. ALL TRENCHING, PIPE LAYING, AND BACKFILLING SHALL BE IN ACCORDANCE WITH FEDERAL OSHA REGULATIONS.
7. CONTRACTOR SHALL MAINTAIN A MINIMUM OF 2 FEET OF COVER FOR ALL UNDERGROUND ELECTRIC AND TELEPHONE UTILITIES.
8. CONTRACTOR SHALL MAINTAIN A MINIMUM OF 4.5 FEET OF COVER FOR ALL WATER DISTRIBUTION PIPING.
9. ALL NEW WATER LINES SHALL BE PRESSURE TESTED AND LEAKAGE TESTED IN ACCORDANCE WITH THE LATEST EDITION OF AWWA STANDARD C600.
10. ALL NEW WATER MAINS SHALL BE DISINFECTED IN ACCORDANCE WITH AWWA STANDARD C651.
11. ALL DOMESTIC WATER AND SANITARY SEWER STUBS TO BE TERMINATED 5 FEET OUTSIDE OF THE BUILDING UNLESS OTHERWISE NOTED. THE END OF THESE SERVICE LINES SHALL BE TIGHTLY PLUGGED OR CAPPED AND MARKED UNTIL SUCH TIME AS CONNECTION IS MADE INSIDE BUILDING BY PLUMBING CONTRACTOR.
12. ALL WORK SHALL COMPLY WITH ALL APPLICABLE CODES, REGULATIONS, AND/OR LOCAL STANDARDS IMPOSED BY LOCAL UTILITY AUTHORITIES.
13. CONTRACTOR SHALL ADJUST LOCATION OF PROPOSED WATER LINES AS REQUIRED TO AVOID CONFLICTS WITH STORM SEWER OR OTHER UTILITIES AT NO EXTRA COST.
14. ALL MATERIAL SHALL BE APPROVED BY THE LOCAL UTILITY COMPANIES UNLESS DIRECTED OTHERWISE BY THE SITE/CIVIL ENGINEER.
15. SHOULD LATENT SOIL CONDITIONS NECESSITATE, CONTRACTOR SHALL INSTALL SPECIAL SUPPORTS FOR PIPING AND/OR APPURTENANCES INCLUDING THE REMOVAL OF UNSUITABLE MATERIAL AND BACKFILLING WITH GRAVEL OR OTHER APPROVED MATERIAL. CONTRACTOR SHALL PERFORM ANY SUCH WORK AS DIRECTED BY THE SITE/CIVIL ENGINEER AND/OR SOILS ENGINEER AT NO COST TO OWNER.
16. V.I.F. = VERIFY IN FIELD
17. PROJECT MUST BE CONSTRUCTED TO ALL WATER COMPANY SPECIFICATIONS.
18. COPPER AND/OR DUCTILE IRON SERVICE LATERAL MATERIAL REQUIRED.
19. GATE VALVES OPEN LEFT.
20. SITE CONTRACTOR IS RESPONSIBLE FOR FURNISHING AND INSTALLING ANY REQUIRED CONNECTORS/ADAPTORS TO CONNECT SITE PIPING/CONDUITS, ETC. TO BUILDING UTILITIES INSTALLED BY OTHERS.
21. EXISTING WATER SERVICES SHALL BE ABANDONED BY TERMINATING AT THE WATER MAIN.

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FFR 25 2026
WATER POLLUTION CONTROL AUTHORITY
BY

NOT FOR CONSTRUCTION, FOR PERMITTING PURPOSES ONLY

Rev. #:	Date	Description

Graphic Scale: 30 0 30 60

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MONROE, CT | W. HARTFORD, CT | NORWOOD, MA
SOLLIENGINEERING.COM
T: (203) 880-5455 | F: (203) 880-9695

Drawn By:	BPD
Checked By:	LAM
Approved By:	KMS
Project #:	24116901
Plan Date:	02/11/26
Scale:	1" = 30'

STATE OF CONNECTICUT KEVIN M. SOLLI 25759 LICENSED PROFESSIONAL ENGINEER
Kevin Solli, P.E.
CT 25759

Project:

PROPOSED REDEVELOPMENT
2135, 2145, & 2155 RESERVOIR AVENUE
TRUMBULL, CONNECTICUT

Sheet Title: ALTERNATE UTILITY PLAN

Sheet #: 2.51

Feb 25, 2026 - 3:10pm bdaak
X:\SE Files\Project Data\2024\24116901 - 2135, 2145, 2155 Reservoir Ave - Trumbull, CT\Engineering Data\CAD Files\24116901 - 2.51 ALT.dwg